Appendix 5.1



Our Ref: GH/Rp/P19012

21st November, 2019

Messrs. M.H.L & Associates Ltd.
Carraig Mor House,
10 High Street,
Douglas Road,
Cork,
Ireland.

Re: Longview, Housing Development, Ballyvolane, Co. Cork. Supplementary Ground investigation, Interpretative report.

Introduction

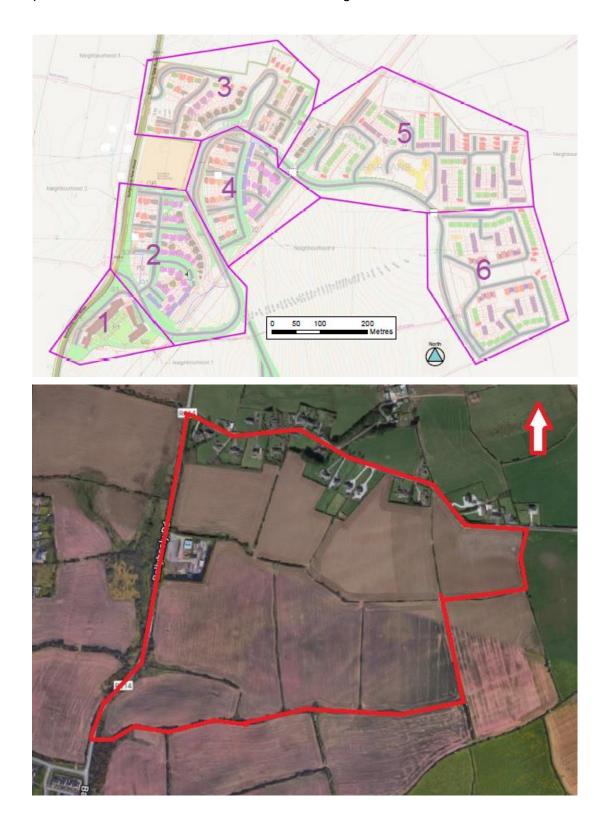
In January 2019, Priority Geotechnical (PGL) were requested by M.H.L & Associates Ltd., Consulting Engineers, acting on behalf of Temporis Ltd. to undertake a supplementary ground investigation at the greenfield site for the proposed housing development at Ballyvolane, Co. Cork. The site is situated immediately off the R614, Ballyhooley New Rd. The site topography was such that is rose from 67mOD to 125mOD. Previously preliminary ground investigation works were carried out by PGL between the 18th of August and the 20th of September, 2017, ref: P17105. The initial phase of the investigation focused on the bedrock profile and rockmass (solid gology) characteristics.

The purpose of this current phase of ground investigations associated with Neighbourhood ¹: 2; was to establish the type, condition and engineering characteristic

P19012_Rp_Int_F01 1 of 34

 $^{^{1}}$ Neighbourhoods: 1, 2, 3, 4, 5 and 6; were investigated to provide data for the entire development.

of the soils (superficial deposits) within the site and assess the potential for re-use of deposits within the earthworks outline and assess groundwater.



P19012_Rp_Int_F01 2 of 34

The scope of the works as defined by MHL in consultation with JBA Consulting² and PGL, comprised of;

- Trial pit excavations;
- Rotary boreholes;
- All associated sampling;
- Standpipe well installations;
- Laboratory testing, including improvement binder trials and
- Associated reporting.

Fieldworks

The fieldwork was carried out by PGL between the 14th and the 19th February, 2019 in general accordance with, BS 5930 (2015) Code of Practice for Site Investigation and Part 9 of BS 1377 (1990), Method of Tests for Soil for Civil Engineering Purposes, *in situ* Tests. This report should be read in conjunction with the exploratory and photographic records and laboratory test data, accompanying this report.

Trial Pits

Twenty four (24) trial pit excavations were dug to 1.8m below existing ground level (bgl) to 3.6m bgl using a 13t tracked excavator. The records accompany this report and are discussed herein.

Location	Final depth, m bgl	Location
TP01	2.10	Moderate
TP02	3.00	Moderate
TP03	3.40	Moderate.
TP04	3.40	Moderate.
TP05	2.90	Moderate.
TP06	2.10	Moderate.
TP07	3.50	Moderate.
TP08	1.80	Moderate.
TP09	3.20	Moderate.
TP10	3.60	Moderate.
TP11	3.00	Moderate.
TP12	2.40	Moderate.

² JBA Consulting are separately providing input into the groundwater assessment for the works.

P19012_Rp_Int_F01 3 of 34

Location	Final depth, m bgl	Location
TP13	2.50	Moderate.
TP14	2.50	Moderate.
TP15	2.90	Moderate.
TP16	2.40	Good.
TP17	2.70	Moderate.
TP18	2.90	Moderate.
TP19	1.90	Moderate.
TP20	2.50	Moderate.
TP21	3.50	Poor.
TP22	3.60	Moderate.
TP23	3.50	Poor.
TP24	3.50	Poor.

Boreholes

Nine (9) rotary boreholes were advanced to depths 3.3m bgl to 9.5m bgl using PGL's Soilmec PSM 8G. The exploratory records are accompanying this report and are discussed herein.

Location	Final depth, m bgl
RC01	6.50
RC02	6.80
RC03	9.50
RC04	6.70
RC05	7.50
RC06	3.30
RC07	4.50
RC08	3.50
RC09	3.00

Sampling

Eighty one (81) bulk disturbed samples (B) and thirteen (13) small disturbed samples (D) and 30.7lin.m of core were recovered from the exploratory excavations in accordance with Geotechnical Investigation and Sampling – Sampling Methods and Groundwater Measurements (EN ISO 22475-1:2006).

P19012_Rp_Int_F01 4 of 34

In-situ testing

Standard penetration test

Fourteen (14) standard penetration tests, N values, were carried out in the rotary boreholes using the 60° solid cone in place of the standard split barrel sampler in accordance with Geotechnical Investigation and Testing, Part 3 Standard penetration test, BS EN ISO 22476-3:2005+A1:2011. The data is presented on the exploratory logs accompanying this report and are discussed herein.

Summary of in situ testing

Туре	Quantity	Remarks
Standard Penetration Test, N _{SPT} values	14Nr.	Nspt ranging from 11 to 36 and refusal >50.

Survey and Drawings

The 'as constructed' exploratory locations were subsequently surveyed using Trimble V8 GPS equipment to the Ordinance Survey, Irish Transverse Mercator (ITM) system of coordinates and elevations to Malin Head datum. These locations are shown on the exploration location plans, attached.

Location	Easting	Northing	Elevation, mOD Malin	Final depth, m bgl	Date dd/mm/yyyy
RC01	568782.200	574961.680	75.070	6.50	16/02/2019
RC02	568904.960	574925.580	85.610	6.80	16/02/2019
RC03	568923.070	575058.720	80.990	9.50	17/02/2019
RC04	569032.550	575141.340	97.040	6.70	17/02/2019
RC05	569140.330	575274.500	111.050	7.50	18/02/2019
RC06	568710.160	574974.340	65.820	3.30	16/02/2019
RC07	568812.580	575111.320	74.030	4.50	17/02/2019
RC08	568954.940	575267.950	88.820	3.50	18/02/2019
RC09	568762.060	575354.430	70.770	3.00	18/02/2019
TP01	568922.660	575175.910	84.980	2.10	14/02/2019
TP02	568962.150	575103.820	86.680	3.00	14/02/2019
TP03	569001.230	574967.860	88.630	3.40	14/02/2019
TP04	568847.830	574964.510	80.100	3.40	14/02/2019
TP05	568929.780	575025.470	81.670	2.90	14/02/2019
TP06	568871.960	574999.200	78.000	2.10	14/02/2019

P19012_Rp_Int_F01 5 of 34

Location	Easting	Northing	Elevation, mOD Malin	Final depth, m bgl	Date dd/mm/yyyy
TP07	568883.970	575141.510	79.640	3.50	14/02/2019
TP08	568760.880	575006.480	69.700	1.80	14/02/2019
TP09	568853.920	575164.990	76.430	3.20	14/02/2019
TP10	568808.170	575193.230	71.020	3.60	14/02/2019
TP11	569021.940	575246.900	98.350	3.00	15/02/2019
TP12	569036.240	575318.650	98.680	2.40	15/02/2019
TP13	568916.600	575422.050	80.100	2.50	15/02/2019
TP14	569029.620	575393.960	94.290	2.50	15/02/2019
TP15	569113.960	575398.390	103.610	2.90	15/02/2019
TP16	569563.360	575058.260	127.270	2.40	15/02/2019
TP17	569676.430	574954.770	127.700	2.70	15/02/2019
TP18	569586.050	575248.560	126.650	2.90	15/02/2019
TP19	569512.200	574921.620	127.700	1.90	19/02/2019
TP20	569446.000	575181.310	128.240	2.50	19/02/2019
TP21	568757.950	575329.560	69.920	3.50	19/02/2019
TP22	568788.880	575278.440	69.380	3.60	19/02/2019
TP23	568807.480	575376.330	70.970	3.50	19/02/2019
TP24	568775.510	575386.330	70.870	3.50	19/02/2019

Laboratory Testing

Laboratory testing was scheduled by PGL on behalf of MHL and carried out in accordance with BS1377 (1990), Methods of test for soils for civil engineering purposes and the ISRM suggested methods for rock characterisation, testing and monitoring. Specialist chemical testing was undertaken by Chemtest Ltd. (UK) on behalf of PGL. The laboratory data accompanied the factual report and were summarised as follows;

SUMMARY OF LABORATORY TESTING

Туре	Nr.	Remarks
Natural Moisture Content	09	9% to 33%
Atterberg Limits		Liquid Limit, LL 22% to 56%
	14	Plastic Limit, PL 13% to 36%
		Plasticity Index, PI 6 to 20
Particle Size Distribution	26	10Nr. hydrometer analysis on fine soils
рH	16	6.2 to 8.0

P19012_Rp_Int_F01 6 of 34

Туре	Nr.	Remarks
Sulphate (water soluble as SO ₄)	16	<0.010g/l
Sulphate (acid soluble)	16	<0.010% to 0.028%
Total Sulphur	16	<0.010% to 0.020%
Initial Consumption of Lime	04	TP02, TP03, TP04 and TP05 (+1%, +1.5% pH 12.4)
Compaction, dry density		TP02 1.5m; TP03 0.5m; TP03 1.5m and TP04 0.5m
moisture content relationship	04	Optimum moisture content 10.3% to 15.3%
		Maximum dry density 1.8Mgm ⁻³ to 2.1Mgm ⁻³
Moisture condition value, MCV moisture content relationship	04	TP02 1.5m; TP03 0.5m; TP03 1.5m and TP04 0.5m
MCV	01	MCV0
California bearing ratio, CBR moisture content relationship	04	TP02 1.5m; TP03 0.5m; TP03 1.5m and TP04 0.5m
CBR	02	CBR0.2% and CBR4.0%
CBR Lime; OPC	07	CBR2.3% to CBR75%

Chemical analysis was also undertaken on the soils post improvement with cement and lime binder additions after a period of 28day curing period.

Туре	Nr.	Remarks
рН	07	9.2 to 12.6
Sulphate (water soluble as SO ₄)	07	0.042g/l to 0.33g/l
Sulphate (acid soluble)	07	0.088% to 0.39%
Total Sulphur	07	0.045% to 0.45%

Ground and groundwater conditions

The full details of the ground conditions encountered are provided for on the exploratory records accompanying this report. The records provide descriptions, in accordance with BS 5930 (2015) and Eurocode 7, Geotechnical Investigation and Testing, Identification and classification of soils, Part 1, Identification and description (EN ISO 14688-1: 2002),— Identification and Classification of Soil, Part 2: Classification Principles (EN ISO 14688-2:2004) and Identification and Classification of Rock, Part 1: Identification & Description (EN ISO 14689-1:2004) of the materials encountered, *in situ* testing and details of the samples taken, together with any observations made during the ground investigation.

P19012_Rp_Int_F01 7 of 34

The ground conditions were such that Topsoil was 200mm to 400mm thick; overlying mixed glacial deposits: slightly sandy gravelly SILT, slightly sandy (slightly) gravelly CLAY, clayey gravelly SAND, (very) silty (very) sandy GRAVEL and (very) clayey sandy GRAVEL with variable Cobble contents; with shallow SANDSTONE/ SILTSTONE was encountered 0.9m below existing ground level (bgl) (TP06) to 4.3m bgl (RC03).

Groundwater conditions observed are those relating to the period of the investigation. The normal duration over which a trial excavation remains open may not allow for low volume flow to ingress in cohesive deposits; borehole casing may cut off low volume flow in cohesive deposits. The groundwater regime should be assessed from standpipe well installations, where available.

Groundwater was encountered within the trial pit excavations or borehole casing during fieldworks at depths between 0.7m bgl and 7.5m bgl. Seasonal variations may occur. Groundwater strikes are summarized as follows:

Location	Groundwater strike, m bgl	Remarks
RC01	1.3	Standpipe installed
RC02	1.5	Standpipe installed
RC03	4.3	Standpipe installed
RC03	7.5	
RC04	3.3	Standpipe installed
RC05	2.5	Standpipe installed
RC06	1.3	Standpipe installed
RC07	3.0	Standpipe installed
RC08	1.5	Standpipe installed
RC09	1.07	Standpipe installed
TP01	-	None encountered.
TP02	-	None encountered.
TP03	-	None encountered.
TP04	-	None encountered.
TP05	-	None encountered.
TP06		None encountered.
TP07	3.5	Slow flow.
TP08	1.8	Slow flow.

P19012_Rp_Int_F01 8 of 34

Location	Groundwater strike, m bgl	Remarks
TP09	1.8	Trickling flow.
TP10	3.6	Slow flow.
TP11	-	None encountered.
TP12	1.5	Slow flow.
TP13	-	None encountered.
TP14	-	None encountered.
TP15		None encountered.
TP16	2.4	Slow flow.
TP17	2.7	Slow flow.
TP18	-	None encountered.
TP19	-	None encountered.
TP20		None encountered.
TP21	0.8	Steady flow.
TP22	2.5	Steady flow.
TP23	1.2	Steady flow.
TP24	0.7	Steady flow at several points between depths shown.
TP24	2.2	Dottroon doptilo onown.

Exploratory trial holes were backfilled with arisings. 50mm diameter HDPE standpipe wells were constructed in all nine (9) rotary boreholes to allow for groundwater monitoring:

Location	Depth, m bgl		Backfill	Pipe diameter,	Pipe type
	from	to		mm	
RC01	0.0	2.5	Bentonite.	50	PLAIN
RC01	2.5	6.5	Gravel.	50	SLOTTED
RC02	0.0	3.0	Bentonite.	50	PLAIN
RC02	3.0	6.0	Gravel.	50	SLOTTED
RC03	0.0	6.0	Bentonite.	50	PLAIN
RC03	6.0	9.0	Gravel	50	SLOTTED
RC04	0.0	2.7	Bentonite	50	PLAIN
RC04	2.7	5.7	Gravel	50	SLOTTED
RC05	0.0	2.0	Bentonite.	50	PLAIN
RC05	2.0	5.0	Gravel.	50	SLOTTED
RC06	0.0	1.3	Bentonite.	50	PLAIN
RC06	1.3	3.3	Gravel.	50	SLOTTED
RC07	0.0	2.5	Bentonite.	50	PLAIN
RC07	2.5	4.5	Gravel.	50	SLOTTED

P19012_Rp_Int_F01 9 of 34

Location	Depth, m bgl		Backfill	Pipe diameter,	Pipe type
	from	to		mm	
RC08	0.0	1.5	Bentonite.	50	PLAIN
RC08	1.5	3.5	Gravel	50	SLOTTED
RC09	0.0	1.0	Bentonite.	50	PLAIN
RC09	1.0	3.0	Gravel	50	SLOTTED

Summary of groundwater monitoring

Location	Groundwater	Groundwater, m bgl					
	strike, m bgl (16 – 18/ 02/2019)	22/03/2019	12/04/2019	dd/mm/yyyy	dd/mm/yyyy	dd/mm/yyyy	
RC01	1.3	4.9	5.85				
RC02	1.5	4.3	Dry				
RC03	4.3	3.4	3.85				
RC04	3.3	4.0	5.1				
RC05	2.5	3.6	Dry				
RC06	1.3	4.3	Dry				
RC07	3.0	Dry	Dry				
RC08	1.5	Dry	Dry				
RC09	1.07	0.0 (GL)	0.3				

GL – existing ground level

To fully assess groundwater and seasonal variations, it is recommended to continue to monitor groundwater between September, 2019 and February, 2020.

P19012_Rp_Int_F01 10 of 34

Geotechnical review

The following geotechnical review provides an overview of the ground conditions identified within the site, along with the general characterisation of the deposits encountered. The following sections should be read in conjunction with the exploratory records, the proposed construction details/ plans and other reporting associated with the proposed development works.

Published Geology

The Geological Survey of Ireland, 1:100,000 mapping (Sheet 25) indicated the geology of the area was characterised by Ballytransa Formation (BS, purple Sandstone and Mudstone) and the Gyleen Formation (Sandstone, Mudstone and Siltstone). The Siltstone/ Mudstone are dominant in these formations. A geological fault is noted to the eastern bound of the site running in an N-S direction. Bedrock outcrop/ sub-crops were noted in the study area. The GSI well data base (well ref: 1407SEW046 and 1407SEW159) identified bedrock 2.4m to 7.6m deep within the study area. Yield was described as poor.

Teagasc sub-soil mapping indicated the superficial deposits in the area were characterised by glacial till derived from Devonian sandstones. The National Aquifer Vulnerability Mapping indicates high to extreme vulnerability in the area. Extreme ratings are likely attributed to shallow depth to bedrock or identified outcropping rock in the study area.

The preliminary ground model (P17105) identified 400mm Topsoil; slightly sandy (slightly) gravelly SILT overlying deposits of firm to stiff (slightly) sandy gravelly SILT and medium dense silty sandy GRAVEL with varying cobble content to depths between 2.0m below existing ground level (bgl) and 3.8m bgl. The SILT was 0.70m to 3.10m thick and the GRAVEL 0.70m to 2.50m thick. Bedrock, SANDSTONE and SILTSTONE was encountered 0.5m bgl to 3.8m bgl. Groundwater was encountered between depths of 1.8m bgl to 3.6m bgl.

P19012_Rp_Int_F01 11 of 34

Ground model

The ground model was such that Topsoil was 200mm to 400mm thick; overlying mixed glacial deposits: firm slightly sandy gravelly SILT, firm slightly sandy (slightly) gravelly CLAY, medium dense to dense clayey gravelly SAND, medium dense to dense (very) medium dense to dense silty (very) sandy GRAVEL and medium dense to dense (very) clayey sandy GRAVEL with variable Cobble contents. The mixed glacial, superficial deposits overlay weak to medium strong SANDSTONE/ SILTSTONE 0.9m below existing ground level (bgl) to 4.3m bgl.

Groundwater was encountered 0.7m bgl and 7.5m bgl (64.52mOD -108.55mOD). It has been assumed that further and more detailed assessment of the groundwater regime and hydrogeology has provided by JBA Consulting.

Geotechnical risk Register

- Re-use of excavated deposits high moisture contents (33%), high plasticity (LL 53%) and high fines content (42% passing 63μm sieve); TP01, TP02, TP09 and TP12:
- 2. SAND deposits have been identified as particular risk TP21 and TP24 where groundwater is also present;
- 3. Groundwater in the excavations, foundations and cuttings; below 2.0m bgl is identified as a particular risk;
- 4. Groundwater in foundation excavations has been identified as a particular risk at RC01, RC06 and TP12 and
- 5. Differential settlement associated with cut-fill earthworks is identified as a particular risk.

The site is classified as geotechnical category GC2 where there is an intent to improve soil stiffness with the addition of lime or lime and ordinary portland cement, OPC binder(s) and excavations exceeding 2.0m.

P19012_Rp_Int_F01 12 of 34

Location	Depth, m bgl	Top of Rock mOD	Stability		ndwater al strike)
	ili bgi	IIIOD		m bgl	mOD, Malin
RC01	6.5	73.77	-	1.3	73.77
RC02	6.8	84.31	-	1.5	84.11
RC03	9.5	76.69	-	4.3	76.69
RC04	6.7	95.54	-	3.3	93.74
RC05	7.5	109.75	-	2.5	108.55
RC06	3.3	64.52	-	1.3	64.52
RC07	4.5	71.53	-	3.0	71.03
RC08	3.5	87.32	-	1.5	87.32
RC09	3.0	-	-	1.07	69.7
TP01	2.1	83.78	Moderate	1	-
TP02	3.0	84.28	Moderate	-	-
TP03	3.4	85.83	Moderate	-	-
TP04	3.4	77.7	Moderate	-	-
TP05	2.9	79.87	Moderate	-	-
TP06	2.1	77.1	Moderate	-	-
TP07	3.5	77.74	Moderate	3.5	76.14
TP08	1.8	68.6	Moderate	1.8	67.9
TP09	3.2	73.53	Moderate	1.8	74.63
TP10	3.6	68.32	Moderate	3.6	67.42
TP11	3.0	-	Moderate	1	-
TP12	2.4	=	Moderate	1.5	97.18
TP13	2.5	78.5	Moderate	-	-
TP14	2.5	-	Moderate	-	-
TP15	2.9	101.86	Moderate	-	-
TP16	2.4	125.27	Moderate	2.4	124.87
TP17	2.7	125.4	Moderate	2.7	125
TP18	2.9	126.25	Moderate	-	-
TP19	1.9	125.8	Moderate	-	-
TP20	2.5	126.14	Moderate	-	-
TP21	3.5	-	Moderate	0.8	69.12
TP22	3.6	65.78	Moderate	2.5	66.88
TP23	3.5	-	Moderate	1.2	69.77
TP24	3.5		Moderate	0.7	70.17
41 2 -T	0.0	-	Moderate	2.2	68.67

P19012_Rp_Int_F01 13 of 34

Geotechnical Category 2 should include conventional types of structure and foundation with no exceptional risk or difficult soil or loading conditions. Designs for structures in Geotechnical Category 2 should normally include quantitative geotechnical data and analysis to ensure that the fundamental requirements are satisfied.

Characteristic properties

Mixed Glacial Deposits

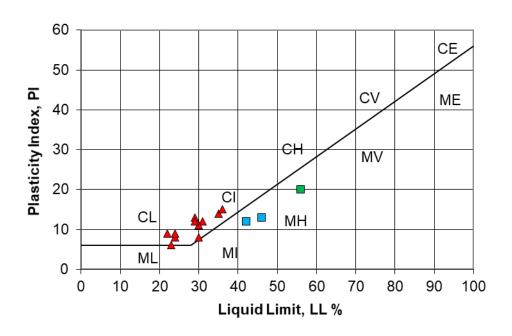
The CLAY deposits encountered were described being of low to intermediate plasticity, (CL/CI) with natural moisture content, *w* ranging between 11% to 17%. The grading analysis for the CLAY indicated a Clay fraction between 26% and 42%; with 25% to 52% Gravel and 22% to 32% Sand fractions.

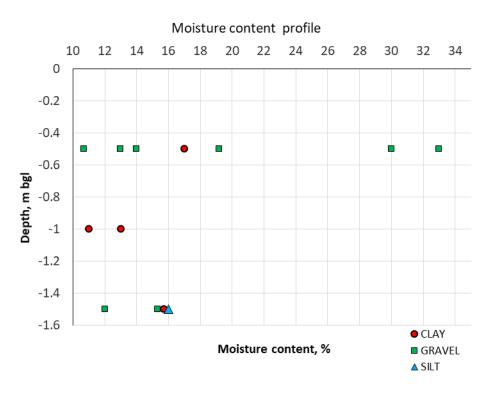
The SILT deposits encountered were described being of intermediate plasticity, (MI) with natural moisture content, *w* ranging 16%. The grading analysis for the SILT indicated a Silt fraction 28%, with 56% Gravel and 17% Sand fractions.

The GRAVEL was characterised by natural moisture content, *w* ranging between 10% and 19% with elevated values of 30% and 33% associated with the shallow high plasticity (MH) silty GRAVEL deposits. The grading analysis or the GRAVEL indicated a Gravel fraction between 42% and 76%; with 9% to 35% Sand and 6% to 22% Silt/ Clay fractions with variable Cobble content 0% to 23%. The grading analysis for the SAND indicated a Sand fraction between 60% and 77%; with 7% to 13% Gravel and 17% to 27% Silt fractions.

P19012_Rp_Int_F01 14 of 34

Summary of plasticity data





In-situ tactile assessment of the cohesive deposits described soft to firm clay soils indicative of an undrained shear strength ranging 40kPa to 150kPa (BS8004, Code of practice for Foundations, 1986).

P19012_Rp_Int_F01 15 of 34

Undrained shear strength was assessed as follows:

taking a range of standard penetration test N_{SPT} = 11 to 20, undrained shear strengths of the order 55kPa to 100kPa are expected for deposits below 1.0m bgl describing firm to stiff CLAY. Elevated N_{SPT} values are attributed to coarse Cobbles particles and more granular deposits.

For Gravels a characteristic N_{SPT}= 17 to 36 for the medium dense to dense deposits, indicated an angle of friction $\phi = 29^{\circ}$ to 36° where;

$$\phi = (12N)^{0.5} + 15$$

Bulk density was determined as follows:

$$\gamma_{sat} = 16.8 + 0.15 N_{60} (kN/m^3)$$

Cohesive:

Granular:
$$Y_{sat} = 16.0 + 0.1N (kN/m^3)$$

The unit weight has been adjusted for bulk density and dry density based on moisture content data.

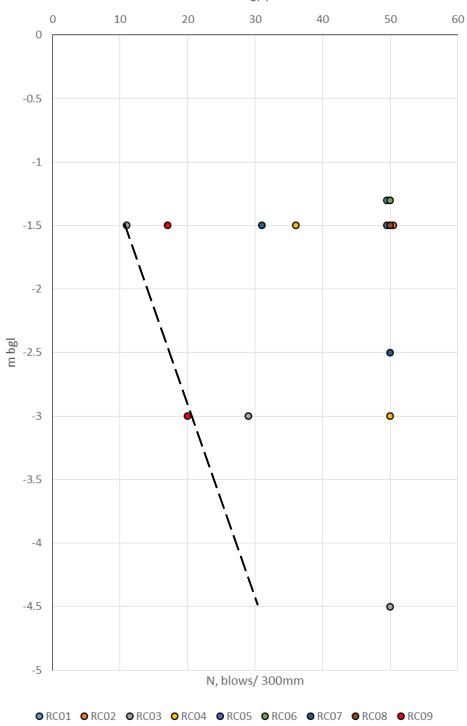
Strata	Characteristic N _{SPT}	Unit weight, kPa	Moisture content, w%	Bulk density, Mgm ⁻³	Dry de Mg	ensity, m ⁻³
CLAY	11	18.45	11 - 17	1.88	1.61	1.69
	20	19.80	11 - 17	2.02	1.73	1.82
GRAVEL	17	17.70	11 – 19 ³	1.80	1.52	1.63
	36	19.60	11 – 19	2.00	1.68	1.80

A bulk factor of 1.35 is recommended for earthworks volumes.

P19012_Rp_Int_F01 16 of 34

³ Elevated moistures 30% and 33% have been omitted from this assessment.

Uncorrected N_{SPT} profile



P19012_Rp_Int_F01 17 of 34

The SANDSTONE bedrock has not been characterised where it has been previously assessed, PGL report ref: P17105: CIV- CIII, poor to fair, weak bedrock. The rotary boreholes have been provided primarily to allow for groundwater monitoring associated with the hydrogeological reporting, by Others.

Foundations

It is recommended to found within weathered bedrock, below depths of 1.0m bgl to 1.6m bgl. It is also recommended to found within the firm cohesive CLAY and medium dense GRAVEL deposits below depths of 1.0m bgl. Shallow strip foundations are considered. Where there is a variation in elevation (topography) stepped foundations shall be considered; in accordance with Diagram 12. Elevation of steeped foundations (TG Doc A, 2012).

A presumed bearing pressure, PABV of 75kN/m² (kPa) to 150kPa is expected of firm CLAY deposits (BS8004 Code of practice for foundations, 1986, Table 1). A presumed bearing pressure of 200kPa to 600kPa is expected of medium dense GRAVEL deposits (BS8004, 1986).

Table 1 — Presumed allowable bearing values under static loading

	se values are for preliminary design purposes only, or the depth of embedment of the foundation (see 2.			ls or downwards. No addition has
Category	Types of rocks and soils	Presumed bearing		Remarks
		kN/m ² *	kgf/cm ^{2 a} tontf/ft ²	
Rocks	Strong igneous and gneissic rocks in	10 000	100	These values are based on
	sound condition	4 000	40	the assumption that the
	Strong limestones and strong	3 000	30	foundations are taken
	sandstones			down to unweathered rock.
	Schists and slates	2 000	20	For weak, weathered and
	Strong shales, strong mudstones and strong siltstones			broken rock,
Non-	Dense gravel, or dense sand and gravel	> 600	> 6	Width of foundation not
cohesive soils	Medium dense gravel, or medium dense sand and gravel	< 200 to 600	< 2 to 6	less than 1 m. Groundwater level
	Loose gravel, or loose sand and gravel	< 200	< 2	assumed to be a depth not less than below the base of
	Compact sand	> 300	> 3	the foundation. For effect
	Medium dense sand	100 to 300	1 to 3	of relative density and
	Loose sand	< 100	< 1	groundwater level,
		Value depend		
0.1	**	degree of loos		
Cohesive	Very stiff boulder clays and hard clays	300 to 600	3 to 6	Group 3 is susceptible to
soils	Stiff clays	150 to 300	1.5 to 3	long-term consolidation
	Firm clays	75 to 150	0.75 to 1.5	settlement (see 2.1.2.3.3).
	Soft clays and silts	<75	< 0.75	For consistencies of clays, see Table 5
	Very soft clays and silts	Not applicabl	e	see Table 5
Peat and	organic soils	Not applicabl	e	i
Made grou	and or fill	Not applicabl	e	Ī.
* 107.25 kN/	$m^2 = 1.094 \text{ kgf/cm}^2 = 1 \text{ tonf/ft}^2$.			

P19012_Rp_Int_F01 18 of 34

Where foundations are to be constructed within the weathered rock mass, BS8004 (1986) identified a presumed bearing value of 2,000 kN/m² for non-weathered strong Sandstone/ Siltstones (Group 4). In accordance with Figure 1 — Allowable bearing pressures for square pad foundations bearing on rock (for settlement not exceeding 0.5% of foundation width) this should be reduced to a value within the range of 100kPa to 250kPa for shallow strip foundations in the bedrock with non-intact fractures fracture spacing 40mm- 200mm. A minimum foundation width; 365mm to 450mm is required where founding in the weathered rockmass (TGD Pt. A; Building Regulations 2012).

A characteristic shear strength of 55kPa (N_{SPT} = 11) is recommended at a depth below 1.0m bgl. Taking a partial factor of safety, 1.4, for assumed founding dimensions yields an ultimate bearing pressure of q_{ult} 235kPa in the firm mixed glacial deposits (D 10.m to 1.3m, B_{min} 0.675m, bearing capacity factor Nc = 6.0, Skempton, 1951).

Fir the granular deposits a characteristic friction of $\phi = 30^{\circ}$ is recommended. Taking a partial factor of safety, 1.25, for assumed founding dimensions yields an ultimate bearing pressure of q_{ult} 210kPa to 235kPa in the medium dense granular deposits (D 1.0m, B_{min} 0.675m, bearing capacity factor Nc = 19.8, N $_{\gamma}$ 6.89 and Nq 9.6, Terzaghi, 1943). Where groundwater is present a depth B below the foundation a reduction in an ultimate bearing pressure to a value of q_{ult} 115kPa, may be expected.

	Top of	Proposed de foundation	pth to	Groundwater			
Location	Rock mOD	mOD, Malin	m bgl	m bgl	mOD, Malin	Strata	PABV, kPa
RC01	73.77	73.77	1.3	1.3	73.77	SILTSTONE	250
RC02	84.31	84.31	1.3	1.5	84.11	SANDSTONE	250
RC03	76.69	79.49	1.5	4.3	76.69	CLAY	75
RC04	95.54	95.54	1.5	3.3	93.74	SANDSTONE	250
RC05	109.75	109.75	1.3	2.5	108.55	SANDSTONE	250
RC06	64.52	64.52	1.3	1.3	64.52	SANDSTONE	250
RC07	71.53	72.53	1.5	3.0	71.03	CLAY	75
RC08	87.32	87.32	1.5	1.5	87.32	SANDSTONE	250
RC09	1	-	-	1.07	69.7	-	-
TP01	83.78	83.78	1.2	-	-	SILTSTONE	250

P19012_Rp_Int_F01 19 of 34

	Top of	Proposed de foundation	pth to	Groundwa	iter		
Location	Rock mOD	mOD, Malin	m bgl	m bgl	mOD, Malin	Strata	PABV, kPa
TP02	84.28	85.38	1.3	-	_	Slightly sandy gravelly SILT	75
TP03	85.83	87.13	1.5	-	-	Sandy very clayey GRAVEL	100
TP04	77.7			-	-	Clayey very sandy GRAVEL	100
TP05	79.87	80.67	1.0	-	-	Slightly sandy gravelly CLAY	75
TP06	77.1	77	1.0	-	-	SILTSTONE	250
TP07	77.74	78.6	1.0	3.5	76.14	Clayey sandy GRAVEL	100
TP08	68.6	68.6	1.1	1.8	67.9	SANDSTONE	250
TP09	73.53	75.4	1.0	1.8	74.63	Slightly sandy gravelly CLAY	75
TP10	68.32	70.0	1.0	3.6	67.42	Slightly sandy gravelly SILT	75
TP11	-	97.5	0.9	-	-	SANDSTONE	250
TP12	_	97.2	1.5	1.5	97.18	Slightly sandy gravelly CLAY	75
TP13	79.1	79.1	1.0	-	-	Clayey sandy GRAVEL	100
TP14	-	93.29	1.0	-	-	Slightly sandy gravelly SILT	75
TP15	101.86	102.61	1.0	-	-	Slightly sandy gravelly SILT	75
TP16	125.27	126.27	1.0	2.4	124.87	Slightly sandy gravelly SILT	75
TP17	125.4	126.7	1.0	2.7	125	Clayey sandy GRAVEL	100
TP18	126.25	125.65	1.0	-	-	SANDSTONE	250
TP19	125.8	127.2	0.5	-	-	GRAVEL	100
TP20	126.14	127.7	0.5	-	-	GRAVEL	100
TP21	-	-	-	0.8	69.12	-	-
TP22	65.78	-	-	2.5	66.88	-	-
TP23	-	-	-	1.2	69.77	-	-
TP24	- TD04 to TD	- 104 and DC00	-	0.7	70.17		-

Locations TP21 to TP24 and RC09 were outside the housing layout in lands on the other side of the R614.

For a characteristic N_{SPT} = 11 in the CLAY/ SILT deposits an allowable bearing pressure up to 75 to 100kPa is expected for an upper limit of 25mm settlement (Terzaghi and Peck, 1967) for a minimum foundation width 675mm.

P19012_Rp_Int_F01 20 of 34

For a characteristic N_{SPT} = 20 in the GRAVELS an allowable bearing pressure up to 100kPa is expected for an upper limit of 25mm settlement for a minimum foundation width 675mm where a factor of 0.5 has been applied to allow for the influence of groundwater (Terzaghi and Peck, 1967).

It is recommended to undertake a plate loading tests at foundation level 1.0m bgl in superficial deposits, to fully assess settlement and bearing capacity of the in situ glacial deposits.

Re-use of deposits

Trial pits TP01 – TP12 form the basis for this current assessment associated with Neighbourhood 2. With plasticity indices, PI 6 to 20 the deposits are at the lower limit of suitability for soil improvement. Ordinary Portland Cement was considered and included in the trial binder additions where the plasticity was low, PI<10. A series of initial consumption of lime, ICL tests were carried out and it was identified that 1% and 1.5% lime ⁴ provided for the minimum required pH 12.4.

A programme of binder additions was detailed to assess soil improvement.

Binder					
Lime, % dry wt.	OPC, % dry wt.				
1.0	2.0				
1.5	-				
1.5	3.0				
2.0	2.0				
3.0	-				

California bearing ratio, CBR was used to determine the degree of improvement. Specimen with varied binder additions were prepared and specimen cured (soaked) for a period of 28 days prior to testing.

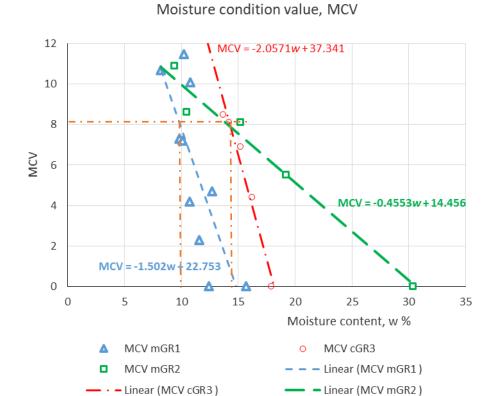
The standard compactive effort associated with the CBR testing was applied (2.5kg drop hammer, 3 layers of 62 blows evenly distributed over each layer).

P19012_Rp_Int_F01 21 of 34

⁴ Clogrennane Lime Ltd, Co. Carlow; CaO Calcium Oxide: Clayfix

An optimum moisture content of 10.3% and 11.0% has been determined for the clayey GRAVEL deposits. Typically compaction 95% maximum dry density (2.0 - 2.1Mgm⁻³) can be achieved within the range of moisture content 9% up to 15%. Some drying -1% may be necessary. A minimum MCV8 is achieved at optimum moisture content to moistures -2% to -4% dry of optimum.

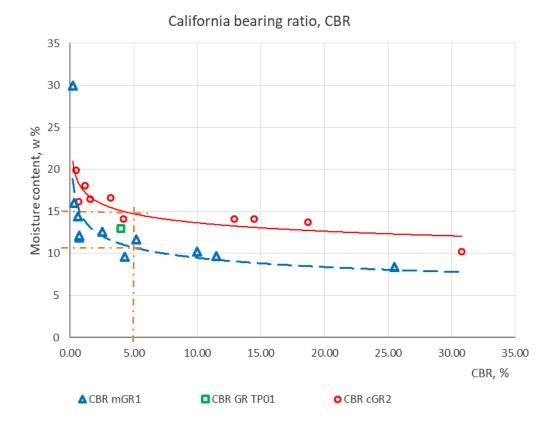
An optimum moisture content of 14.6% and 15.3% has been determined for the silty GRAVEL deposits with higher fines contents. Typically compaction 95% maximum dry density (1.8 - 2.1Mgm⁻³) can be achieved within the range of moisture content 8% up to 19%. A minimum MCV8 is achieved at moisture content -2% dry of optimum.



MCV8 is achieved a natural moisture content 10% to 15%. Typically drying of -1% to -4% is required for the clayey GRAVEL, cGR to be suitable for re-use as general cohesive fill. It is expected to be achievable by stockpiling and air drying with the exception of the upper silty GRAVEL, mGR of high plasticity (TP02 0.5m).

P19012_Rp_Int_F01 22 of 34

A minimum MCV8 is achieved at moisture content -2% to -4% dry of optimum.



The CBR data has been summarised and group based on optimum moisture content and maximum dry density data mGR1 (TP02 and TP04; optimum moisture content 10.3% - 11.0% 2.0Mgm⁻³ to 2.1Mgm⁻³) and cGR2 (TP03; optimum moisture content 14.6% - 15.3% 1.8Mgm⁻³ to 1.9Mgm⁻³). A CBR5% is achieved at moisture content below 11% and 15%.

The re-compacted deposits associated with the propose earthworks outline were assessed using the following relationship(s);

$$q_{ult}$$
 (kPa) = (CBR% -1.5) x 65 or
 q_{ult} (kPa) = CBR% x 70;
 c_u (kPa) = CBR% x 23 (TRRL 889, 1979);
 Log_{10} c_u (kPa) = 0.82 + 0.126 x MCV (Davitt, 1989);
 Log_{10} CBR(%) = -0.65 + 0.140 x MCV;

P19012_Rp_Int_F01 23 of 34

In general for a California bearing ratio >CBR2.0%, an undrained shear strength of greater than 50kPa is expected with equivalent Moisture condition values >MCV8 and standard penetration test N_{SPT} >10.

Processing of excavated deposits shall be by a combination of air drying for the granular deposits and improvement with Lime binder for the cohesive deposits; to provide a minimum re-compacted CBR5%.

$$q_{ult}$$
 227 $kPa = (5.0\% -1.5) \times 65$
 q_{ult} 350 $kPa = 5\% \times 70$
 c_u 115 $kPa = 5 \times 23$ (TRRL 889, 1979)
 c_u 75 $kPa = 5 \times 15$ (PGL, 2019)

Location	Depth, m bgl	Natural moisture	CBR _{natural} ,	Binder % dry		CBR _{binder} ,	Strata	CBR change, CBR% per %	Air drying,
		content, w%	70	Lime	OPC	70		binder	w % change
TP01	0.5	13	4.0	,	_	-	Silty GRAVEL	-	-4
TP02	0.5	30	0.2	,	-	-	Very silty	-	-18
TP12	0.5	30	0.2 *	3	-	2.3	GRAVEL	0.7	-10
TP02	1.5	16	0.3	1.5	-	8.5	SILT	5.5	-5
TP03	0.5	19	1.2	,	-	-	Very silty GRAVEL	-	-4
	1.5	16	1.6	1	2	35	Clayey GRAVEL	11.1	-2
TP04	0.5	14	0.7	1.5	-	15	Clayey GRAVEL	9.5	-5
TP05	0.5	17	1.0 **	1	2	29		9.3	-7
				2	2	49	CLAY	12.0	-1
	1.0	11	4.0 **	1.5	3	76		16	-1

^{*} Data referenced TP02 0.5m

To allow for variance, an additional +0.5% Lime binder is provided for in the design binder. No % increase is required for OPC binder additions, where required.

It is recommended to provide for the addition of 2% Lime to improve the excavated deposits to provide a design CBR5% for foundations. It is undetermined the %binder required to improve the high plasticity (MH) deposits where binder addition 3% Lime achieved CBR2.3%. It isanticipated that a % OPC may be necessary, possibly 2%.

^{**} Data referenced TP02 1.5m

It is recommended to provide for the addition of 2% Lime to improve the excavated deposits to provide a design CBR5% for ground bearing floor slab with the exception of the high plasticity deposits.

A target MCV10 is recommended for basic quality control.

It is recommended to undertake a plate loading test at formation level to fully assess settlement and bearing capacity (California bearing ratio, CBR) of the recompacted (improved) deposits where associated with the earthworks outline.

Soil improvement and Quality control procedures

A site trial and programme of quality control will be required.

- Prior to improvement, it shall be confirmed that the deposits to be improved (feed stock) are the same as those deposits assessed as part of the ground investigation and where any variance in moisture content, plasticity and sulphates are identified; the appropriate revisions to the proposed % binder reviewed/applied.
- The natural moisture content of each volume of earthworks shall be determined to allow determination of the %binder where a value of 2% Lime is based on the soil dry weight (2% Lime is 20g Lime per 1kg dry soil).
- A site trial shall be undertaken or demonstration area prepared (minimum of 500m²) to assess design binder content, the spread rate and bearing capacity provided by the minimum design CBR. The mixing, curing (mellowing) period, pulvarisation and compactive methods shall be assessed for their effectiveness.
- The formation shall be adequately compacted prior to placing improved soils.
 The binder shall be applied and the rate of spread checked and adjusted as necessary.
- Check the mix-depth and adjust as necessary. The mix depth shall not exceed 300mm.

P19012_Rp_Int_F01 25 of 34

- A minimum curing time of 4 hours is recommended for the Lime binder before the compactive effort is applied.
- Moisture condition value, MCV target MCV10 shall be determined prior to application of the compactive effort for each area up to 500m².
- Check the degree of pulvarisation prior to application of the compactive effort. It
 may be necessary to apply further mixing prior to compaction. A minimum of six
 (6) passes of a vibratory smooth wheeled roller of minimum weight 4300kg per
 meter width shall be applied where the grading analysis identified Class 1
 general cohesive deposits (Specification for roadworks, Table 6/4).
- A specimen shall be collected and prepared for California bearing ratio, CBR testing allowing for a period of 7 days curing in the soaked condition to verify the deign CBR.
- A plate loading test shall be carried out to assess the degree of compaction and in situ CBR for each compacted layer.

The above points are provided as a guidance to aid in the specification of the soil improvement. A Specialist Contractor shall be engaged to undertaken the works to provide a suitable programme of soil improvement to provide for a minimum design CBR5%. A design binder addition of +2% Lime has been recommended.

It may be require to provide an additional 2% OPC to improve high moisture content high plasticity deposits.

P19012_Rp_Int_F01 26 of 34

Hard standing

California bearing ratio, CBR of CBR0.2% to CBR4.0% in the upper layers 0.5m bgl.

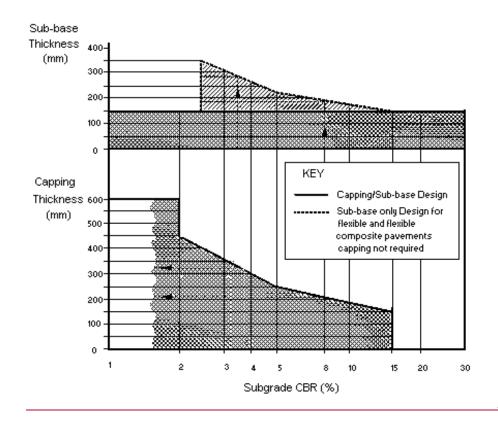
Location	Depth, m bgl	Natural moisture content, w %	CBR _{natural} , %	Capping thickness, mm	Sub-base thickness, mm
TP01	0.5	13	4.0	300	150
TP02	0.5	30	0.2	600	150
TP12	0.5	30	0.2	600	150
TP03	0.5	19	1.2	600	150
TP04	0.5	14	0.7	600	150
TP05	0.5	17	1.0	600	150

Capping 600mm thick and a sub-base thickness of 150mm is recommended for hard standing and pavement construction in accordance with Tii DMRB Vol 7 Pt 2A, TD25-26/1- Figure 4.1.

The improvement of the formation using +2.0% lime binder may also be considered to provide CBR15% without a capping requirement in the GRAVEL deposits. Alternatively using +1.5% lime binder may also be considered to provide CBR8% with a capping requirement 200mm and 150mm sub-base in the more cohesive deposits, subject to finish road levels and the requirement to maintain an earthworks balance within the site.

Drainage shall be provide a minimum 600mm below the underside of capping to maintain equilibrium CBR.

P19012_Rp_Int_F01 27 of 34



Groundwater

Groundwater was encountered 0.7m bgl and 7.5m bgl (64.52mOD -108.55mOD). It has been assumed that further and more detailed assessment of the groundwater regime and hydrogeology has provided by JBA Consulting.

Groundwater was encountered, however seasonal variations may occur. Groundwater in cuttings 2.0m is identified as a design risk.

Particle size d_{10} ranged from 0.212mm to 0.02mm for the GRAVEL deposits, indicative of permeabilities within the range 4.5×10^{-4} ms⁻¹ to 4.0×10^{-6} ms⁻¹, describing medium to low permeability.

Infiltration viability may be given full consideration where an infiltration coefficient of magnitude 10⁻⁵ ms⁻¹ or greater exists (SUDS Manual C753, 2015). Soakaways may be considered as a form of drainage control within the site where further investigation is undertaken at the proposed locations.

P19012_Rp_Int_F01 28 of 34

It is recommended that soakaways are lined with a non-woven geotextile separator to prevent immigration of fines and an associated loss of capacity.

Alternative forms of surface water control such as oversized pipework and swails shall be considered. Detention or attenuation of surface water and rainfall may be considered as a means of control and provision of a non-potable water supply (harvesting).

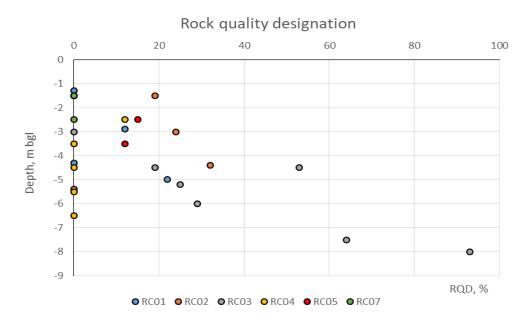
Excavations

Excavations shall be by means of hydraulic excavator in the superficial deposits. Trial pit stability was described as moderate to good with poor stability at TP21 to TP24 where SAND was encountered. Excavations into the weathered rockmass up to a depth 2.5m were achieved using a 13t tracked excavator.

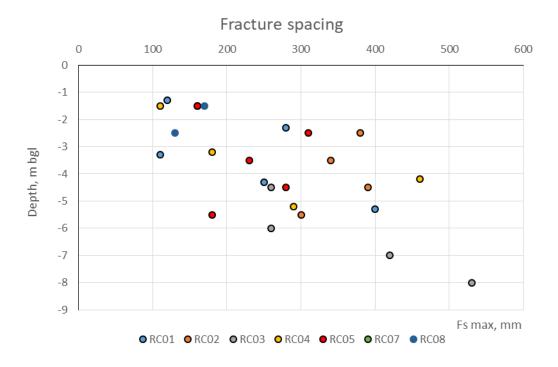
Location	Depth of excavation, m bgl	Stability	Depth to rock, m bgl
TP01	2.1	Moderate	1.2
TP02	3.0	Moderate	2.4
TP03	3.4	Moderate.	2.8
TP04	3.4	Moderate.	2.4
TP05	2.9	Moderate.	1.8
TP06	2.1	Moderate.	0.9
TP07	3.5	Moderate.	1.9
TP08	1.8	Moderate.	1.1
TP09	3.2	Moderate.	2.9
TP10	3.6	Moderate.	2.7
TP11	3.0	Moderate.	-
TP12	2.4	Moderate.	-
TP13	2.5	Moderate.	1.6
TP14	2.5	Moderate.	-
TP15	2.9	Moderate.	1.75
TP16	2.4	Good.	2.0
TP17	2.7	Moderate.	2.3
TP18	2.9	Moderate.	0.4
TP19	1.9	Moderate.	1.9
TP20	2.5	Moderate.	2.1
TP21	3.5	Poor.	-
TP22	3.6	Moderate.	3.6
TP23	3.5	Poor.	-
TP24	3.5	Poor.	-

P19012_Rp_Int_F01 29 of 34

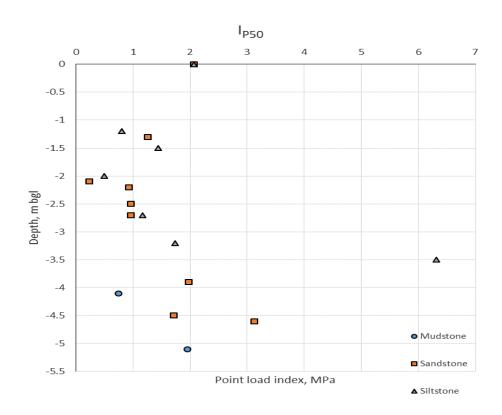
Rock quality designation typically<25% for the weak to medium strong rockmass.

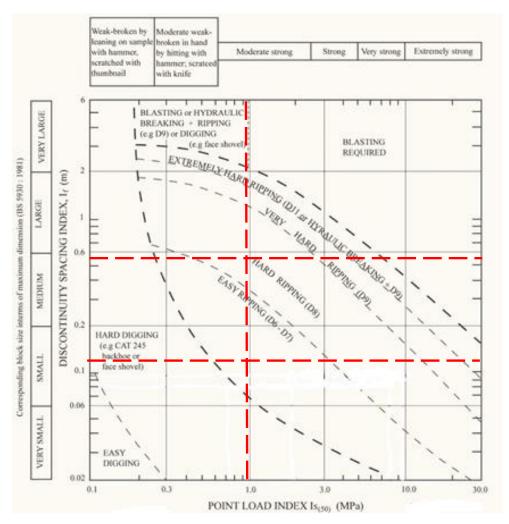


With maximum fracture spacing ranging between 110mm to 530mm where strenght is assumed characterised by point load index I_{P50} 0.96MPa to 2.1MPa (P17105) excavation can be expected tobe by means of hydraulic ripping and breaking.



P19012_Rp_Int_F01 30 of 34





Provisionally, a cut angle of 1H:1.5V is recommended where it can be expected that cuttings will be compound in superficial deposits and the weathered rock mass. No further assessment is provided at this time.

Chemical

Based on the pH (6.2 to 8.0) and sulphate (<0.010g/l and <0.010% to 0.028%) data indicate design sulphate class DS-1 in accordance with BRE Digest for concrete in aggressive ground for static groundwater conditions. With pH 6.2 (PH5.5- 6.5) acidic conditions were identified. The use of acid resistant cement is required.

It is expected to interact with the bedrock within the earthworks outline. With Total Sulphur <0.010% to 0.20% in the bedrock, there is no pyrite content (TS 0.3% to 1.0% I.S. 398 Part 1); sulphates were below the limit 0.2%.

P19012_Rp_Int_F01 32 of 34

Location	TP01	TP02	TP02	TP02	TP03	TP04	TP04
Top depth, m bgl	1.5	0.5	1.5	2.5	1.5	0.5	2.5
Moisture, %	14	21	11	14	11	9.6	9.2
рН	7.2	6.5	6.4	6.2	6.6	7.0	7.3
Sulphate (2:1 Water Soluble) as SO4, g/l	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010
Total Sulphur, %	< 0.010	0.013	< 0.010	0.013	< 0.010	< 0.010	< 0.010
Sulphate (Acid Soluble), %	0.028	0.022	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010

Location	TP05	TP06	TP09	TP09	TP09	TP19
Top depth, m bgl	1.0	1.0	1.0	2.0	2.9	0.6
Moisture, %	8.1	14	11	11	8.8	12
рН	7.5	7.2	7.0	7.1	7.5	7.1
Sulphate (2:1 Water Soluble) as SO4, g/l	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010
Total Sulphur, %	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	0.016
Sulphate (Acid Soluble), %	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	0.020

Location	TP21	TP22	TP24	TP24
Top depth, m bgl	1.0	1.5	1.0	2.5
Moisture, %	9.0	8.4	14	19
рН	7.9	8.0	7.5	7.7
Sulphate (2:1 Water Soluble) as SO4, g/l	< 0.010	< 0.010	< 0.010	< 0.010
Total Sulphur, %	< 0.010	0.015	0.020	< 0.010
Sulphate (Acid Soluble), %	< 0.010	< 0.010	0.014	< 0.010

Post improvement chemistry was reviewed.

Location	TP02	TP03	TP04	TP05	TP05A	TP05B	TP12
Top depth, m bgl	1.50	1.5	0.50	1.0	0.5	0.5	0.50
Moisture, %	11	12	8.8	10	11	11	23
рН	9.2	12.3	11.2	12.5	12.4	12.6	12.2
Sulphate (2:1 Water Soluble) as SO4, g/l	0.29	0.13	0.33	0.051	0.071	0.042	0.12
Total Sulphur, %	0.19	0.048	0.071	0.069	0.051	0.045	0.45
Sulphate (Acid Soluble), %	0.28	0.13	0.12	0.15	0.091	0.088	0.39

The sulphate values post improvement were >0.24% requiring an increase in design sulphate class to DS-2. The no special requirements with regard to concrete mix design other than the use of acid resistant cement in untreated deposit

P19012_Rp_Int_F01 33 of 34

Should there be any queries in relation to the data collected or subsequent analysis

please do not hesitate to contact our office.

Yours sincerely,

For Priority Geotechnical,

Greg Hayes CEng BE MEngSc MIEI

Geotechnical Specialist

No responsibility or liability can be held by PGL for ground conditions between exploratory locations. The exploratory logs provide for ground profiles and configuration of strata relevant to the investigation depths achieved during the fieldworks. Caution shall be taken when

extrapolating between such exploratory locations.

The scope of the works has been defined by others. Additional works and further observations may form part of a more detailed investigation. This report may be subject to change where

further information becomes available.

No account has been taken of potential subsidence or ground movement due to mineral extraction, mining works or karstification below or in proximity to the site, unless specifically

addressed.

This report has been prepared for Employer and their Representative as outline, herein. The information should not be used without their prior written permission. PGL accepts no responsibility or liability for this document being used other than for the purposes for which it was intended.

KEY TO SYMBOLS ON EXPLORATORY HOLE RECORDS

All linear dimensions are in metres or millimetres

DESCRIPTIONS

** Drillers Description
Friable Easily crumbled

SAMPLES

U() Undisturbed 102mm diameter sample, () denotes number of blows to drive sampler

U()F, U()P F- not recovered, P-partially recovered
U38 Undisturbed 38mm diameter sample

P(F), (P) Piston sample - disturbed
B Bulk sample - disturbed
D Jar Sample - disturbed

W Water Sample

CBR California Bearing Ratio mould sample
ES Chemical Sample for Contamination Analysis

SPTLS Standard Penetration Test S lump sample from split sampler

CORE RECOVERY AND ROCK QUALITY

TCR Total Core Recovery (% of Core Run)

SCR Solid Core Recovery (length of core having at least one full diameter as % of core run)

RQD Rock Quality Designation (length of solid core greater than 100mm as % of core run)

Where there is insufficient space for the TCR, SCR and RQD, the results may be found in the remarks column

If Fracture Spacing in mm (Minimum/Average/Maximum) NI - non intact, NR - no recovery

AZCL Assumed Zone of Core Loss

NI Non intact

GROUNDWATER

abla Groundwater strike

▼ Groundwater level after standing period

Date/Water Date of shift (day/month)/Depth to water at end of previous shift shown above the date

and depth to water at beginning of shift given below the date

INSITU TESTING

S Standard Penetration Test - split barrel sampler
C Standard Penetration Test - solid 60° cone

SW Self Weight Penetration

Ivp, HVp (R) In Situ Vane Test, Hand Vane Test (R) demonstrates remoulded strength

K(F), (C), (R), (P) Permeability Test
HP Hand Penetrometer Test

MEASURED PROPERTIES

N Standard Penetration Test - blows required to drive 300mm after seating drive

x/y Denotes x blows for y mm within the Standard Penetration Test

x*/y Denotes x blows for y mm within the seating drive

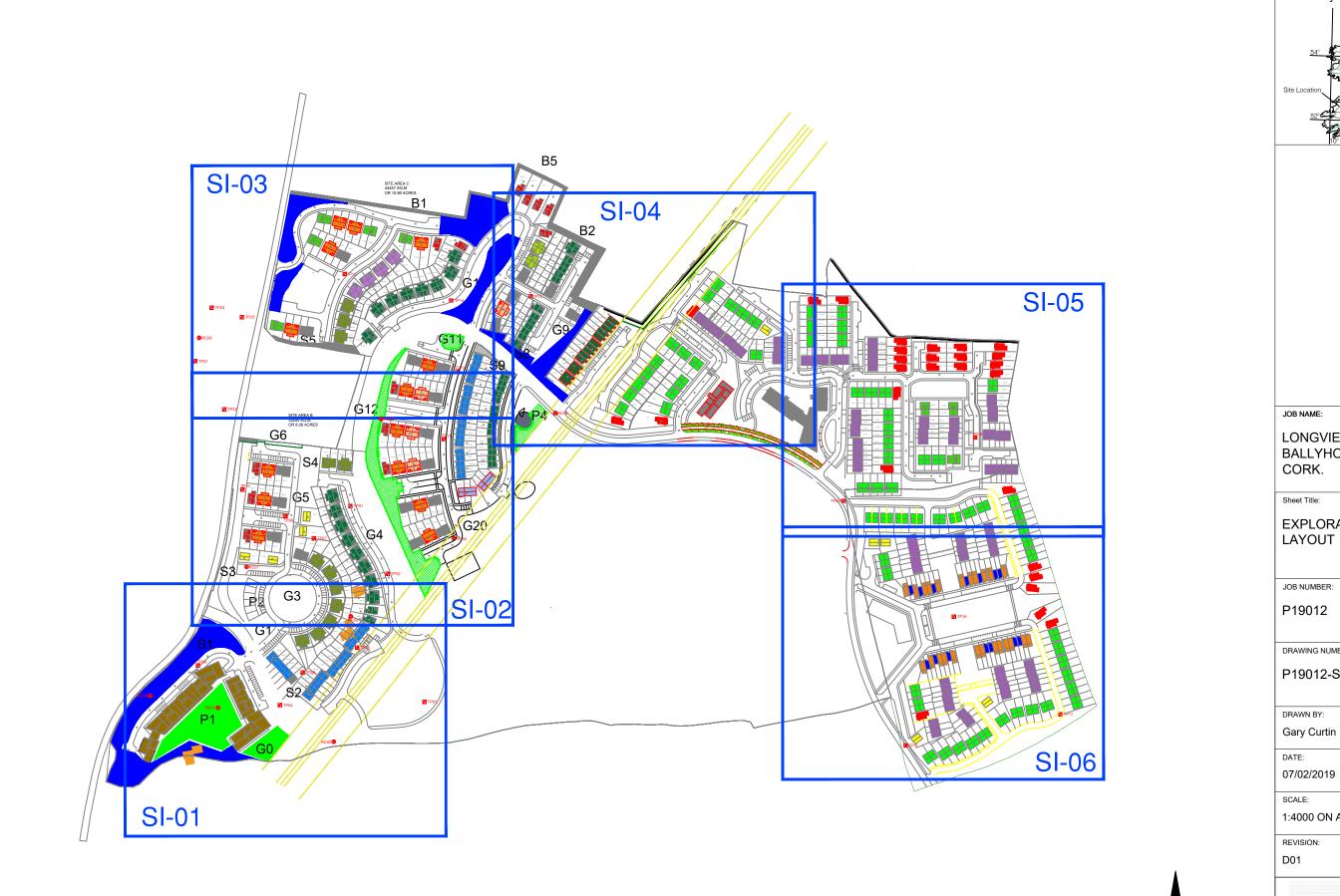
C_{II} Undrained Shear Strength (kN/m²)

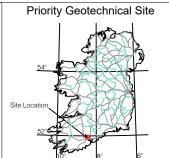
CBR California Bearing Ratio

ROTARY DRILLING SIZES

Index Letter	Nominal Diameter (mm)			
	Borehole	Core		
N	75	54		
Н	99	76		
Р	120	92		
S	146	113		







LONGVIEW, BALLYHOOLY RD, CORK.

EXPLORATORY LOCATION LAYOUT

JOB NUMBER:

P19012

DRAWING NUMBER:

P19012-SI-A

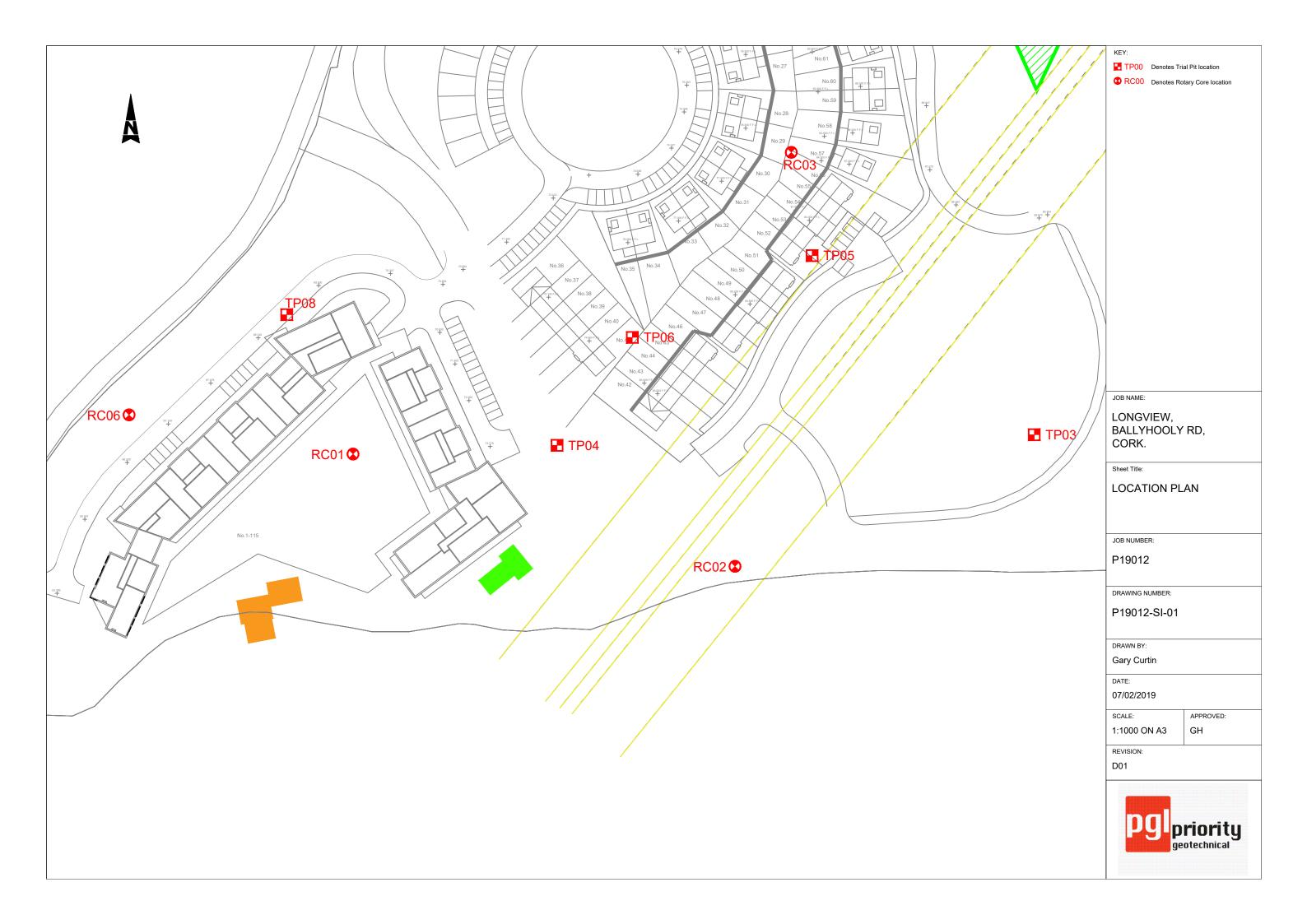
Gary Curtin

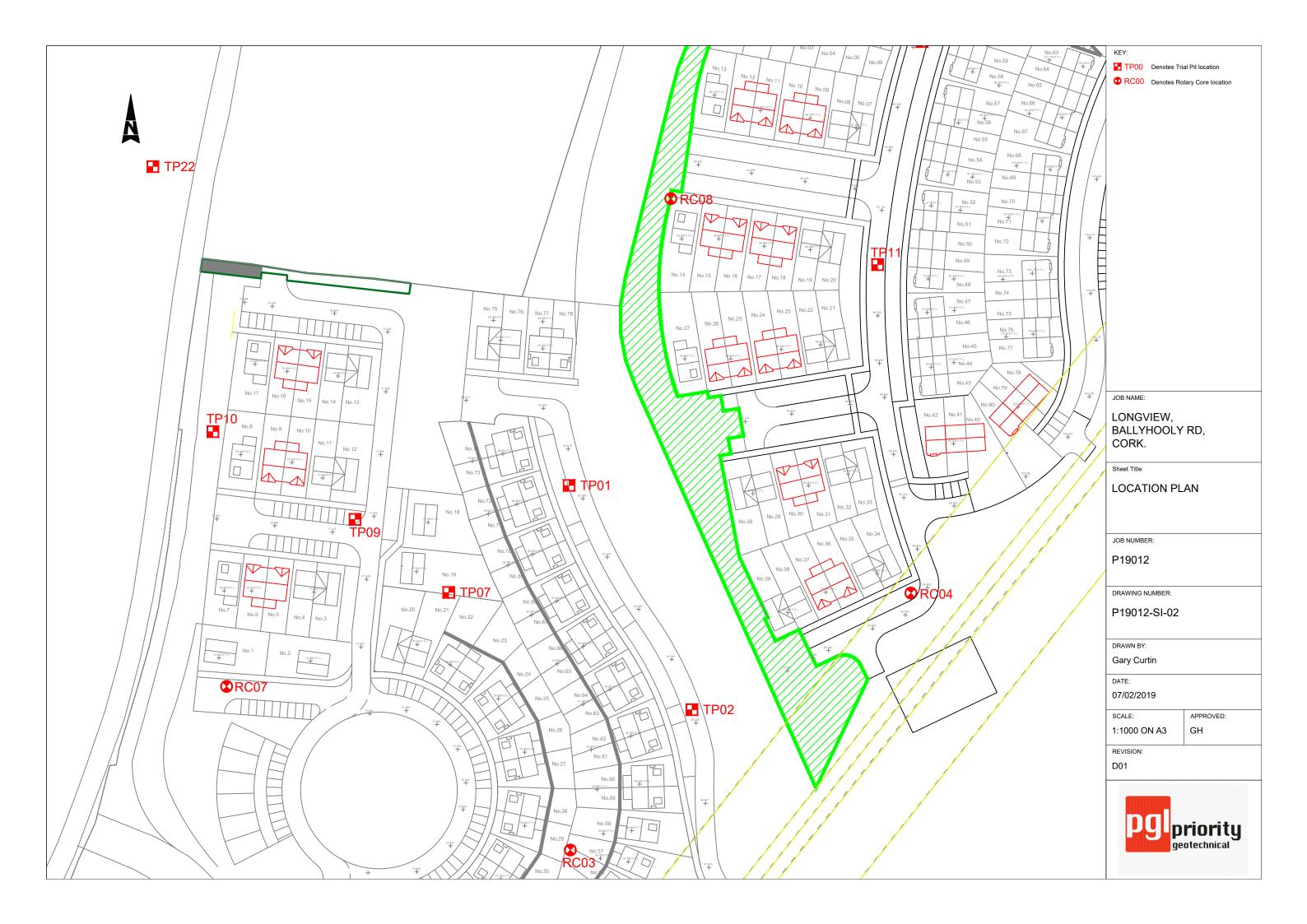
1:4000 ON A3

GH

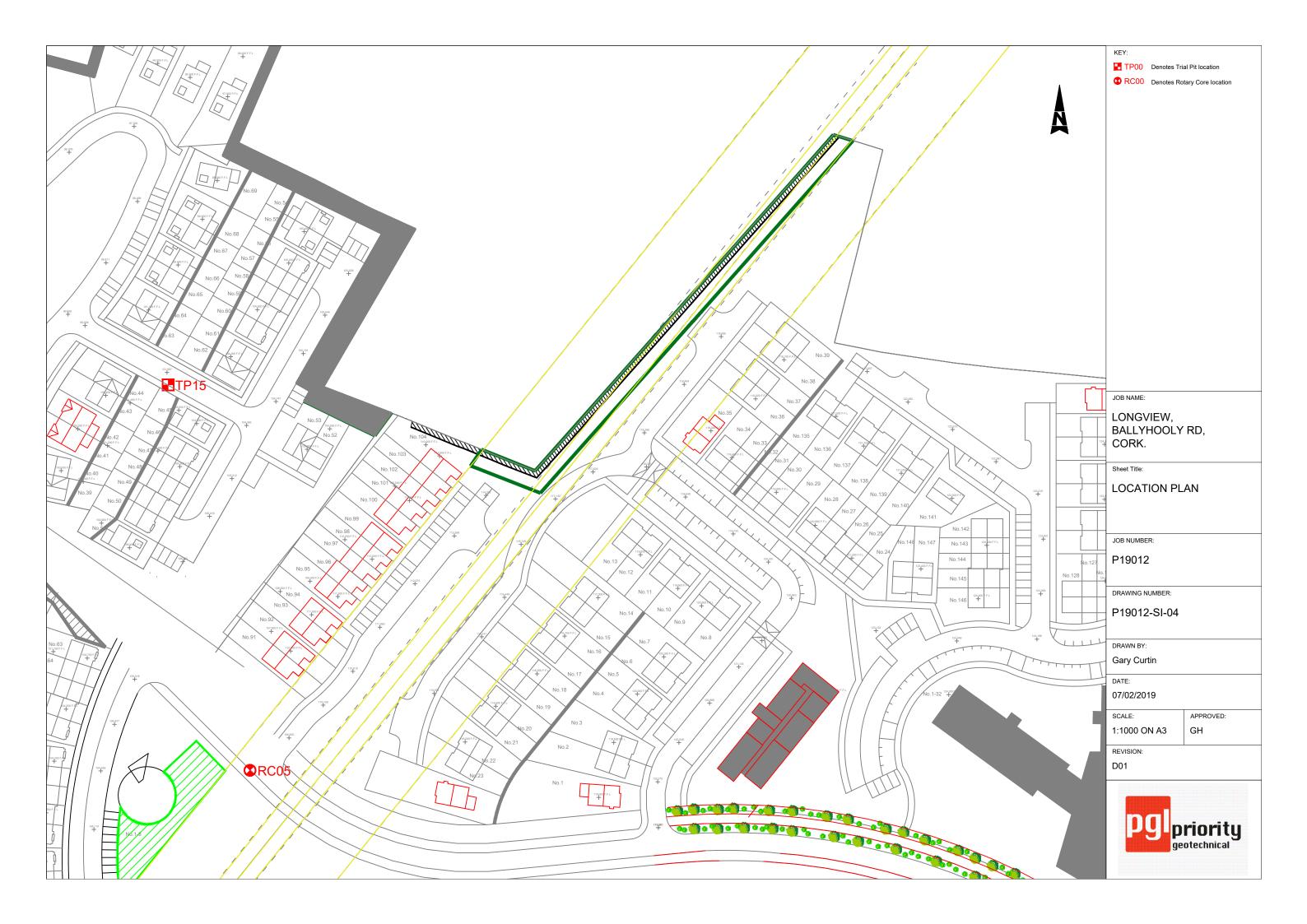
APPROVED:

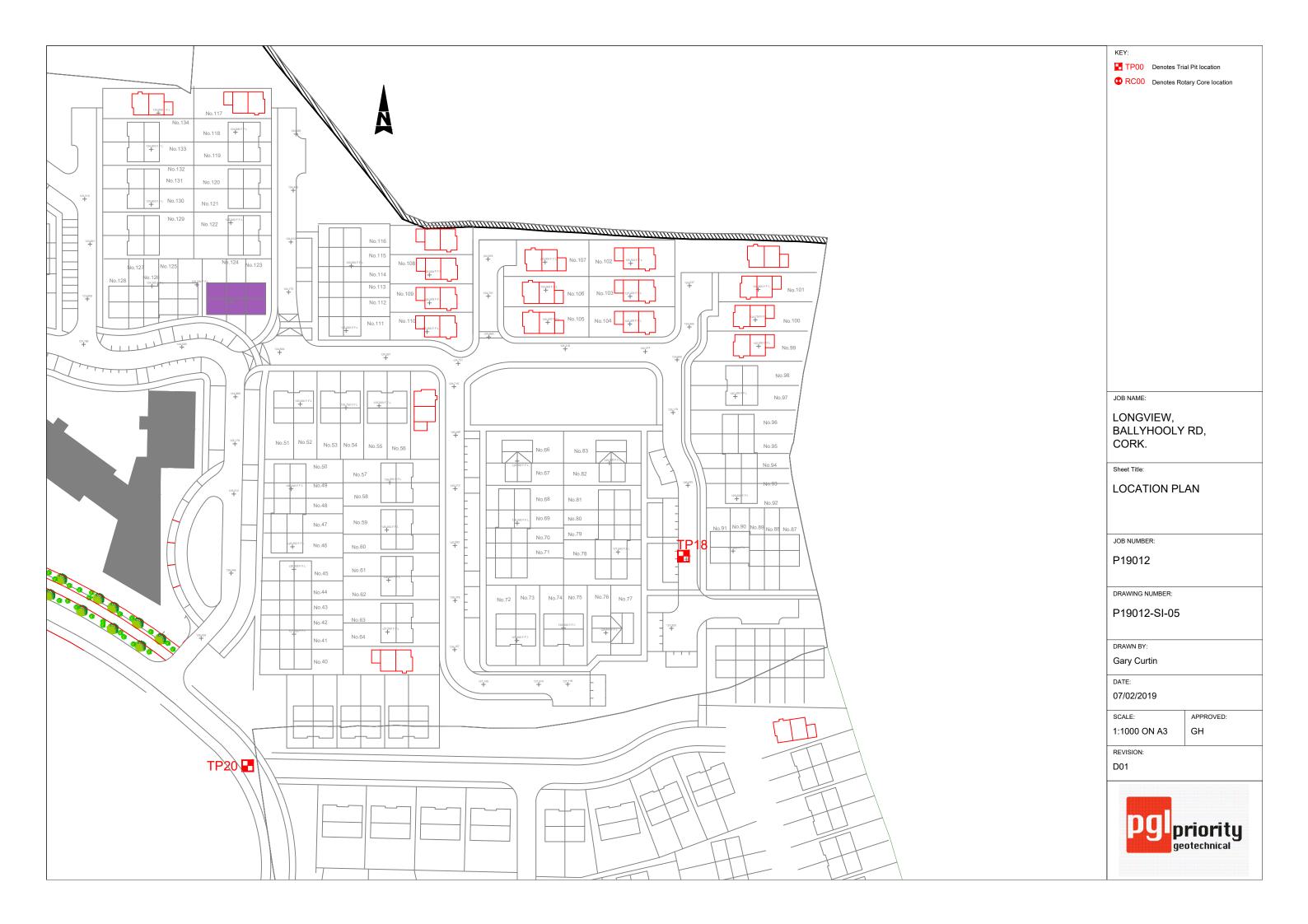














Project Id:	P19012	Title:	Site Plan	
Project Title:	Longview Developments	Scale:	1:3500	
Location:	Cork	Engineer:	MHL Consulting Engineers	
				1



Legend Key

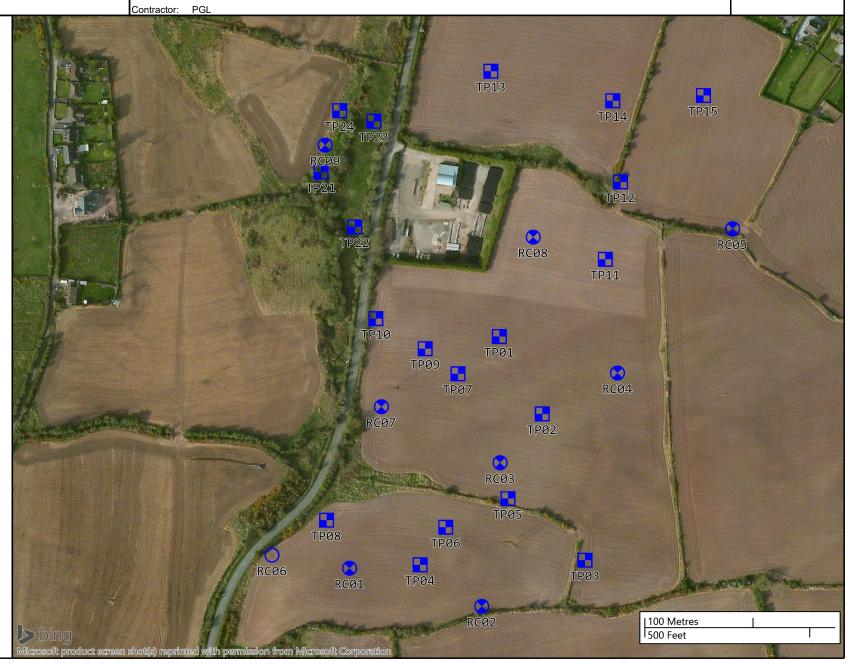
Client:

O Locations By Type - Empty

Locations By Type - RC

Locations By Type - RO

Locations By Type - TP



Project Id:	P19012	Title:	Site Plan	
Project Title:	Longview Developments	Scale:	1:3500	ı
Location:	Cork	Engineer:	MHL Consulting Engineers	



Legend Key

Client:

O Locations By Type - Empty

Locations By Type - RC

O Locations By Type - RO

Locations By Type - TP



þ	priori	ty			Tel: Fax	: 021 4 : 021 4	echnical L 1631600 1638690 eotechnica					GW gged By:	Borehole N RC01 Sheet 1 of	
Proje	ct Name	Longvie Develo	ew Housin				ct No.		Co-ords:	5687	782E - 57		Hole Typ	е
Locat	ion:		lane, Cork			P 190	12		Level:	75.0°	7m OD		Rotary core	eu
Clien	·-	Tempor							Dates:	16/0)2/2019		1:50	
Well	Water	Depth	Type /Fs (min,	Co	ring (%)	Depth (m)	Leve		10/0		atum Descrip		
	50 (25 for 10mm/50 for 0mm) (C) 1.30 - 2.90 4.30 - 5.00 5.00 - 6.50		5mm 120mm 20mm 5mm 280mm 45mm 10mm 40mm	63 71 100	24 34 93	0 12	1.30 40/m 10/m	73.77		Lith purp Sar Wea diss	ology: Meple, SILTS athering: Vathered with solution.	dium weak to m TONE with com inds. Veathered to sli th oxidation and set dipping circa	nedium strong, nmon ightly I minor 60 degrees	3 3 5 1 5 1 1 1 1 1 1 1
Struck (I	1.30		5mm 400mm 20mm		Comr See shift		6.	th (m bg 50		bgl)	ing Dia (mm 131 Shift	Equipment: Method:	Soilmec PSM Compressed a	
	terminat	ed at 6.5m b onse zone f					Shift D	ata:	1.3	10	Shift 6/02/2019 08 6/02/2019 18	:00 0.00	Start of shift End of boreho	







Longview, Ballyhooly Rd Cork P19012 Project RC01 Number:

Project No Engineer

þ	prior	rity _{ical}			Tel Fax	: 021 4 : 021 -	echnical Li 4631600 4638690 eotechnica				Log	led By: GW ged By: KH	RC02 Sheet 1 of	2
Proje	ct Nam	e: Longvie	w Housin		•		ct No.		Co-ords:	5689	905E - 574		Hole Typ Rotary cor	е
Loca	tion:		ane, Cork				<u>. – </u>	I	Level:	85.6	1m OD		Scale 1:50	-
Clien	t:	Tempori	s Ltd.					ı	Dates:	16/0	02/2019		16/02/2019	
Well	Water Strike (m)	Depth (m)	Type /Fs (min, max, avg)	Cc TCR	ring ((%)	Depth (m) / FI (/m)	Leve (mOE		k	Strat	tum Descript	ion	
		3.00 - 4.40 4.40 - 5.40	10mm 160mm 70mm 80mm 120mm 15mm 390mm 160mm 60mm	100	82 86 95	19 24 32	1.40 1.50 10/m	84.21 84.11		Slict	en hole bori sumed Siltst hology: Purp NDSTONE i d Siltstone le athering: SI solution app earing along	ng. Driller desc gravelly Clay. ng. Driller desc one lithology. le, fine to medi with minor quar enses. ightly weathere arent and mino g fracture surfact acture sets obse	ribed: Rock. um grained tz veining d with minor r clay ces. erved.	1 2 3 4 5 6 7 8 9
	ndwate			1		l	Hole In			ı		Equipment:	Soilmec PSM	
	m bgl) R d 1.50	ose to After (mi	n) Sealed		Comr See shi		Hole Dept) Hole Dia (r 76	nm) Cas	sing Dia (mm) 131	Method:	Compressed a	air mist.
	termina	ated at 6.8m b ponse zone fr					Shift D	ata:	Groundwater (1	Shift 6/02/2019 08:0 6/02/2019 18:0		Remarks Start of shit End of boreh	







Longview, Ballyhooly Rd Cork P19012 Project RC02 Number:

Project No Engineer

þ	prior i	ty			Tel Fax	: 021 4 :: 021 4	echnical L 1631600 4638690 eotechnica					GW Iged By:	RC03	3
Proje	ct Name	Longvi Develo	ew Housin pment	g		Proje P1901	ct No. 12	(Co-ords:	5689	923E - 57	5059N	Hole Typ Rotary cor	
Locat	ion:	Ballyvo	lane, Cork					I	Level:	80.9	9m OD		Scale 1:50	
Clien	t:	Tempor	ris Ltd.					I	Dates:	17/0	02/2019		17/02/2019	
Well	Water Strike (m)	Depth (m)	Type /Fs (min, max, avg)	Co TCR	ring SCR	(%)	Depth (m) / FI (/m)	Leve (mOE			Stra	atum Descript	ion	
	Y	N=11 (3,3/2,3,2,4) (C) N=29 (3,4/6,6,8,9) (C) 50 (25 for 40mm/50 for 70mm) (C) 4.50 - 6.00 7.50 - 8.00	10mm 260mm 60mm 120mm 120mm 340mm 320mm 330mm 280mm	100	53 93 100	19 29 64 93	4.30 4.50 10/m	76.69 76.49		Opp Ass Litt SAl We smi Fra plai	en hole bor sumed San nology: Mec NDSTONE athering: S earing alon nor oxidatio actures: Dip nar rough f	ring. Driller descriptions of gravelly Clay. Tring. Driller descriptions of gravelly Clay. Tring. Driller descriptions of gravelly Clay. Tring. Driller descriptions of gravelly weathere an colouration. Tring 45 to 60 descriptions of gravelly weathere surfaces are spacing.	ribed: Rock. ple, se lenses. d with clay ces and	1 — 2 — 3 — 4 — 5 — 6 — 7 — 9 — 9 — 9 — 9 — 9 — 9 — 9 — 9 — 9
Struck (ı	ndwater m bgl) Ros 4.30 7.50	: se to After (m	nin) Sealec		Com ı See sh		Hole Ir) Hole Dia (mi 76		sing Dia (mm 131	wethou.	Soilmec PSM Compressed	air mist.
	termina	ted at 9.5m l oonse zone f					Shift D	ata:	Groundwater (m	1	Shift 7/02/2019 08: 7/02/2019 18:		II) Remarks Start of shi End of boreh	ft.

pg	priority				Tel: Fax:	021 4 021 4	echnical Li 1631600 1638690 eotechnica								RC03	3
Proje	ct Name:	Longvie	ew Housing				ct No.		Cd	o-ords:	5689	923E - 575	KH 5059	N	Sheet 2 of Hole Typ Rotary col	е
Locat	ion:		lane, Cork			F 190	12		Le	vel:	80.9	9m OD			Scale 1:50	eu
Client	t:	Tempor	is Ltd.						Da	ites:	17/0	02/2019			17/02/2019	
Well	Water Strike (m)	Depth (m)	Type /Fs (min, max, avg)	Co TCR	ring (%) RQD	Depth (m) / FI (/m)	Leve (mOl		Legend	,	Stra	atum	Descripti	on	
			max, avg)		SCK SCK	RQD	9.50	71.49			SAI We sme min Fra plai	NDSTONE athering: S earing alon for oxidatio ctures: Dip nar rough f dium fractu	with slightly grade from color pring fracture sping sping fracture sping	45 to 60 de ire surfaces	d with clay es and grees with and close to	10
Struck (r	ndwater: m bgl) Rose 1 4.30 7.50	to After (m	in) Sealed		Comn See shif		Hole In			On: Hole Dia (mr	m) Cas	sing Dia (mm	.⊢	uipment: thod:	Soilmec PSM Compressed	
	terminated ed. Respon						Shift D	ata:	G	7.5	1	Shift 7/02/2019 08: 7/02/2019 18:	:00	Hole Depth (m bgl 0.00 9.50	Remarks Start of sh End of borel	ift.







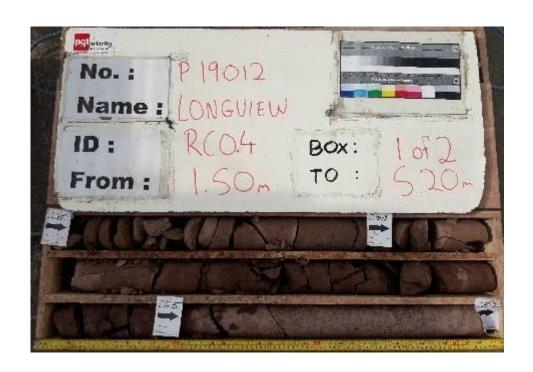
Number: RC03

Project Project No Engineer Longview, Ballyhooly Rd Cork P19012

oject No P1901

				Pr	iority	Geote	chnical L	td.				Dr	illed	Ву:	Borehole N	0.
pg	prior	ity		-	Tel	: 021 4	631600 4638690							_	RC04	.
	geotechni	cal		ww			a638690 eotechnica	al.ie				LO	gged KH	d By:	Sheet 1 of	
Proje	ct Name		w Housin			Proje	ct No.		Co-c	ords:	5690)33E - 57			Hole Typ	е
		Develop				P190 ⁻	12	\dashv					• • •		Rotary cor Scale	ed
Locat	ion:	Ballyvol	ane, Cork						Leve	el:	97.0	4m OD			1:50	
Client	::	Tempori	s Ltd.						Date	es:	17/0	2/2019		,	17/02/2019	
Well	Water Strike (m)	Depth (m)	Type /Fs (min, max, avg)	TCR	ring ((%) RQD	Depth (m) / FI (/m)	Leve (mOE		.egend		Str	atun	n Descripti	on	
		N=36 (4,7/7,8,10,11) (C) 1.50 - 3.00 50 (25 for Omm)(C) 3.00 - 4.50 4.50 - 5.20	7mm 110mm 35mm 5mm 180mm 50mm 12mm 290mm 160mm	47 97 100	39 93 100	0 0 53	1.50 10/m 10/m 8/m 6.70	95.54			Lith fine min We cold frac	ology: Me to mediunter to mediunter clay bathering: Nouration and cture surfactures: 1 so planar fracting.	diumm grands. Weath of classes diacetur	Driller descrivelly Clay. In weak to meained SANDS thered witth of ay smearing. In pping circa 6 are surfaces and orehole at 6.700	dium strong, STONE with xidation along 0 degrees nd close	1 2 3 4 5 6 7 8 9
Groui	ndwater	:					Hole In	forma	ition	1:			Ec	uipment:	Soilmec PSM	
	n bgl) Ro 3.30	se to After (mi	in) Sealed		Comr See shi		Hole Dep 6.	th (m bg l 70	l) Ho	ole Dia (mn	n) Cas	sing Dia (mn	¹) Me	ethod:	Compressed a	air mist.
install	termina ed. Res	ted at 6.7m b ponse zone fr ied out in bor	om 2.7m				Shift D	ata:	Grou	undwater (m t 3.3	0	Shift 7/02/2019 18 7/02/2019 08		Hole Depth (m bgl) 6.70 0.00	Remarks End of boreh Start of shi	







Longview, Ballyhooly Rd Cork P19012 Project RC04 Number:

Project No Engineer

þ	prio	rity _{lical}			Tel Fax	: 021 4 :: 021 4 orityge	echnical L 1631600 4638690 eotechnica				Logg	ed By: GW ged By: KH	Borehole N RC05 Sheet 1 of	1
Proje	ct Nam	e: Longvie Develor	w Housin oment	g		Proje P1901		(Co-ords	: 569	9140E - 575	274N	Hole Typ Rotary cor	
Locat	tion:	Ballyvol	ane, Cork					I	Level:	111	.05m OD		Scale 1:50	
Clien	t:	Tempori	is Ltd.					I	Dates:	18	/02/2019	,	18/02/2019	
Well	Water Strike (m)	Depth (m)	Type /Fs (min, max, avg)	Co TCR	ring ((%)	Depth (m) / FI (/m)	Leve (mOE		nd	Strat	um Descripti	on	
		50 (25 for Omm/50 for Omm) (C) 1.50 - 2.50 2.50 - 3.50 4.50 - 5.50 5.50 - 6.50	15mm 160mm 80mm 10mm 120mm 120mm 120mm 120mm 160mm	100	40 80 71 78	0 0 0 0	1.30 1.50 10/m 10/m 10/m	109.75		O A Li le le W ar ar sp	pen hole boringsumed Sand thology: Mediral Sand Sand thology: Mediral Sand iron oxidation actures: Heathactures: Heathactures sacing, 60 deguigh fractures	ng. Driller descr stone lithology. um strong, purp STONE with mir eathered with cl on colouration. vily fractured wi gree dipping and	ibed: Rock. ile, fine nor clay ay smearing th close d planar	1 2 3 4 5 6 7 8 9
Struck (ı	ndwate m bgl) Ro 2.50	r: ose to After (mi	in) Sealec		Comr See shi		Hole Ir			a (mm) C	asing Dia (mm) 131	Equipment:	Soilmec PSM Compressed a	
	termina	ated at 7.5m b sponse zone fr					Shift D	ata:	Groundwate 2.5		Shift 18/02/2019 08:00 18/02/2019 18:00		Remarks Start of shi End of boreh	ft







Project Longview, Ballyhooly Rd Cork Number: **RC05**

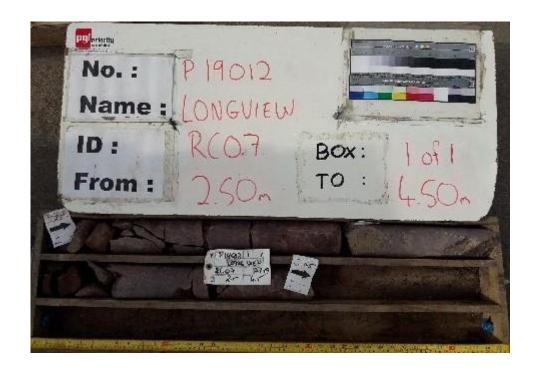
Project No P19012

Engineer MHL Consulting Engineers

pg	priori geotechnic	ty			Tel: Fax	: 021 4 : 021 4	echnical L1 1631600 4638690 eotechnica					Log	GW	Ву:	RC06	5
Proje	ct Name	Longvie Develo	ew Housing				ct No.		Co-	ords:	5687	710E - 574			Hole Typ Rotary open	е
Locat	ion:		lane, Cork			F 190	12		Lev	rel:	65.8	2m OD			Scale 1:50	TIOIC
Client	t:	Tempor	is Ltd.						Dat	es:	16/0	02/2019			16/02/2019	
Well	Water Strike (m)	Depth (m)	Type /Fs (min,		ring (Depth (m) / FI (/m)	Leve		Legend		Stra	tun	Descripti	ion	
	The (III)	50 (25 for 70mm/50 for 0mm) (C)	max, avg)	TCR	SCR	RQD	3.30	64.52 62.52			Slig	en hole bor phtly sandy en hole bor sumed San	ring. grav	Driller desci velly Clay.	ribed:	1 2 3 4 5 6 7 8 7 8 7 8 7 7 7 7
	oundwater:						h	•-					T-		a	9 -
Struck (r		: se to After (m	nent ft data.	Hole In Hole Dept	h (m bg		n: Hole Dia (mn 100	n) Cas	sing Dia (mm 131	\vdash	uipment: thod:	Soilmec PSM Compressed				
	terminat	ted at 3.3m boonse zone f			Shift D	ata:	Gre	oundwater (m i	1	Shift 6/02/2019 08: 6/02/2019 18:		Hole Depth (m bg 0.00 3.30	Remarks Start of sh End of boreh	ift		

þ	priori geotechnic	cal		ww	Tel Fax	: 021 4 :: 021 4 orityge	echnical L 1631600 1638690 eotechnica						G۷	d By:	RC07	7 f 1
Proje	ct Name	: Longvie Develo	ew Housing pment	9		P1901	ct No. 12		Со	-ords:	5688	813E - 57	511	1N	Rotary co	
Locat	ion:	Ballyvol	lane, Cork						Le	vel:	74.0	3m OD			Scale 1:50	
Clien	t:	Tempor	is Ltd.						Da	tes:	17/0	2/2019			17/02/2019	
Well	Water Strike (m)	Depth (m)	Type /Fs (min, max, avg)	Co TCR	ring ((%) RQD	Depth (m) / FI (/m)	Leve (mOl		Legend		Stra	atun	n Descripti	ion	
		N=31 (3,5/8,6,8,9) (C) 50 (25 for 20mm/50 for 40mm) (C) 2.50 - 3.50	max, avg)	70	75	15	2.50	71.53	3		Lith grai min Wea cold bed	ology: Me ined SANI erals. athering: \ ouration ar iding plane ctures: 2 s	dium DST0 Weat and m ess.	Driller descrivelly Clay. In weak, purply DNE with mind thered with one inor dissolution observed. In weak, purply DNE with mind the with mi	le, medium nor pyrite xidation ion along	3 4 5 7 8 9
Grou	ndwater	:					Hole In							quipment:	Soilmec PSM	
	m bgl) Ro s 3.00	se to After (m	in) Sealed		Comr See shi		Hole Dep	t h (m bg 50	jl) l	Hole Dia (mn 76	n) Cas	i ng Dia (mm 131	¹) M €	ethod:	Compressed	air mist.
	termina	ted at 4.5m b ponse zone f					Shift D	ata:	Gi	3.0	1	Shift 7/02/2019 08 7/02/2019 18		Hole Depth (m bgi 0.00 4.50	Remarks Start of sh End of borel	ift

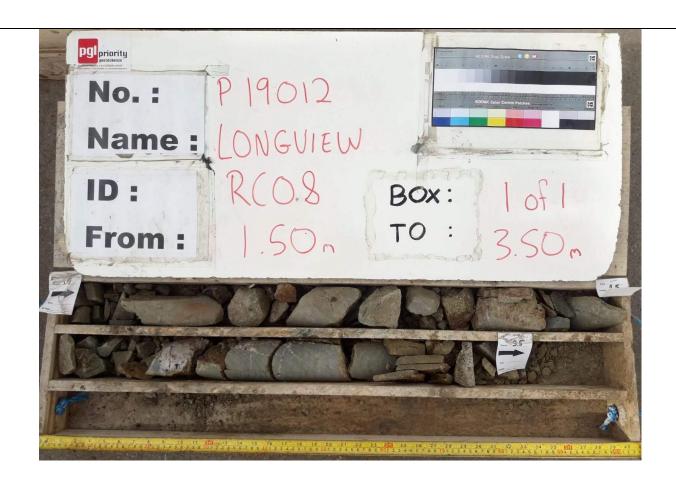




Project Project No Engineer Longview, Ballyhooly Rd Cork P19012 RC07 Number:

þ	prior	cal		ww	Tel: Fax	: 021 4 : 021 4 orityge	chnical L 1631600 1638690 cotechnica						G۷	l By:		RC08 Sheet 1 of	}
Proje	ct Name	: Longvie Develop	w Housin oment	g		Proje	ct No. 12		Co-d	ords:	5689	955E - 57	5268	3N		Hole Typ Rotary cor	
Locat	ion:	Ballyvol	ane, Cork						Leve	el:	88.88	2m OD				Scale 1:50	
Clien	t:	Tempori	s Ltd.						Date	es:	18/0	2/2019			1	8/02/2019	
Well	Water Strike (m)	Depth (m)	Type /Fs (min, max, avg)	Co TCR	ring (%) RQD	Depth (m) / FI (/m)	Leve (mOl		egend		Stra	atun	n Descr	iptio	n	
		2.50 - 3.50	4mm 170mm 60mm 5mm 130mm 70mm	80	43	0	1.50 10/m 10/m 3.50	87.32	2		Lith grai We cold	athering: V ouration th ctures: He	een, graeen, googleen, goo	medium DNE with hered wi hout.	to coa quar tth iro	arse tz veining. n oxide oughout.	1 2 3 4 5 6 7 8 9
Grou	ndwater	:					Hole In							uipmer	nt:	Soilmec PSM	
	m bgl) Ro 1.50	se to After (mi	in) Sealed		Comr See shi		Hole Dep	th (m bg 50	gl) Ho	ole Dia (mn 76	n) Cas	ing Dia (mm 131) Me	thod:		Compressed	air mist.
	termina	ted at 3.5m b ponse zone fr				pe	Shift D	ata:	Grou	undwater (m t	1	Shift 8/02/2019 08 8/02/2019 18		Hole Depth 0.00 3.50	ا ا	Remarks Start of shi End of boreh	ift.





Number:

RC08

Project Project No Engineer

Longview, Ballyhooly Rd Cork P19012

pg	priori	ty			Tel Fax	: 021 4 : 021 4	echnical L 4631600 4638690 eotechnica				Log	GW ged By:		Borehole No RC09 Sheet 1 of	
Proje	ct Name	Longvie Develo	ew Housing		•		ct No.		Co-ords:	5687	762E - 575			Hole Type Rotary core	е
Locat	tion:		lane, Cork					ı	Level:	70.7	7m OD			Scale 1:50	
Client	t:	Tempor	ris Ltd.					ı	Dates:	18/0)2/2019		18	3/02/2019	
Well	Water Strike (m)	Depth (m)	Type /Fs (min, max, avg)	Cc TCR	ring ((%)	Depth (m) / FI (/m)	Leve (mOE			Stra	tum Desci	riptio	n	
		N=17 (2,3/3,5,4,5) (C) N=20 (3,4/4,5,5,6) (C)	max, avg)	TCK	SCR	RQD	3.00	69.97 67.77		Slig	phtly sandy en hole bor avel.	ing. Driller d peaty Clay. ing. Driller d	escrib	ed: Sandy	3
Groui	ndwater	:					Hole Ir	nforma	tion:			Equipme	nt:	Soilmec PSM	6
	m bgl) Ros 1.07	se to After (m	nin) Sealed		Com r See shi		Hole Dep	th (m bgl 00) Hole Dia (mn 131	n) Cas	sing Dia (mm) 131	Method:		Compressed a	ir mist.
	termina	ted at 3.0m boonse zone f					Shift D	ata:	Groundwater (m i	1	Shift 8/02/2019 08: 8/02/2019 18:)	Remarks Start of shi End of boreho	



Priority Geotechnical Ltd. Tel: 021 4631600 Fax: 021 4638690 www.prioritygeotechnical.ie Trial Pit No **TP01**

Sheet 1 of 1

Date

Project Longview Housing Development Name:

Project No. P19012

Co-ords:568923E - 575176N

Level: 84.98m OD

14/02/2019 Scale

Location: Ballyvolane, Cork

Remarks: Trial pit terminated at 2.10m bgl due to rock.

Arisings

Plant: Backfill: Dimensions (m):

3.00

2.00

1:25 Logged

lient:	Temporis L	.td.			1:25 Depth: 2:10m BGL DMC				
Water Strike & Backfill	Samples & In Situ Testing Depth Level							<u> </u>	
	Depth (m)	Туре	Results	(m)	Stratum Description				
				0.35	84.63		(TOPSOIL): Soft brown slightly sandy slightly gravelly SILT. Sand is fine to coarse. Sand is fine to coarse. Gravel is fine to coarse, sub-angular. Orange brown silty sandy GRAVEL with low Cobble		
	0.50 - 1.00 0.50 - 1.00	B D					content. Gravel is fine to coarse, angular.	1	
	1.50 - 2.00	В		1.20	83.78	X X X X X X X X X X X X X X X X X X X	Green SILTSTONE bedrock dipping 75 degrees, striking E-W. Recovered as: Angular and tabular COBBLES.		
				2.10	82.88	* * * * * * * * * * * * * * * * * * *	End of Pit at 2.100m] -	
ability:	Moderate 13T track mach				1	Groundwa	ater: None encountered.		







Number:

TP01

Project Project No Engineer





Number:

TP01

Project Project No Engineer



Priority Geotechnical Ltd. Tel: 021 4631600 Fax: 021 4638690 www.prioritygeotechnical.ie Trial Pit No **TP02**

Sheet 1 of 1

Project Longview Housing Development Name:

Project No. P19012

Co-ords:568962E - 575104N **Level:** 86.68m OD

Date 14/02/2019

Location: Ballyvolane, Cork

Dimensions (m):

3.00

Scale 1:25

Client: Temporis Ltd. Depth: 3 00m BGI

2.0	Logged
	DMC

Temporis L						3.00m BGL DMC
	les & In Si	tu Testing	Depth	Level	Legend	Stratum Description
Depth (m)	(m)	(m OD)	Legena			
0.50 - 1.20 0.50 - 1.20	B D		0.30	86.38	* * * * * * * * * * * * * * * * * * *	TOPSOIL. Soft orange brown slightly sandy gravelly SILT. Sand is fine to coarse. Gravel is fine to coarse, sub-angular.
1.50 - 2.20 1.50 - 2.20	B D		1.30	85.38		Stiff brown slightly sandy gravelly SILT with high cobble content. Sand is fine to coarse. Gravel is fine to coarse, sub-angular to rounded. Cobbles are 63mm to 200mm dia, sub-angular to rounded, Limestone.
2.50 - 3.00	В		2.40	84.28		SANDSTONE. Recovered as brown green blocky Sandstone Cobbles and Boulders
			3.00	83.68		End of Pit at 3.000m
						4
						5
	Samp Depth (m) 0.50 - 1.20 0.50 - 1.20 1.50 - 2.20 1.50 - 2.20	Samples & In Si Depth (m) Type 0.50 - 1.20 B 0.50 - 1.20 D 1.50 - 2.20 B 1.50 - 2.20 D	Samples & In Situ Testing Depth (m) Type Results 0.50 - 1.20 B 0.50 - 1.20 D 1.50 - 2.20 B 1.50 - 2.20 D	Depth (m) Type Results 0.30	Depth (m) Type Results Depth (m) Level (m OD)	Depth (m) Type Results Depth (m) Level (m OD)

Backfill: Arisings.

Remarks: Trial pit terminated at 3.00m bgl due to rock.







Number:

TP02

Project Project No Engineer







Number:

TP02

Project Project No Engineer



Priority Geotechnical Ltd. Tel: 021 4631600 Fax: 021 4638690 www.prioritygeotechnical.ie Trial Pit No **TP03**

Sheet 1 of 1

Project Longview Housing Development Name:

Project No. P19012

Co-ords:569001E - 574968N **Level:** 88.63m OD

Date 14/02/2019 Scale

Location: Ballyvolane, Cork

Dimensions (m):

3.00 1:25

Client: Temporis Ltd. Depth:

Logged

lient:	Temporis L	td.					3.40m BGL DMC	
Water Strike & Backfill		les & In S	itu Testing	Depth	Level	Legend	Stratum Description	
	Depth (m)	Туре	Results	(m) (m OD) Leg		Logona		
	0.50 - 1.20 0.50 - 1.20 0.50 - 1.20	В В D		0.30	88.33		(TOPSOIL). Orange, brown, soft, very silty very sandy GRAVEL. Gravel is fine to coarse.	
	1.50 - 2.50 1.50 - 2.50 1.50 - 2.50	B B D		1.20	87.43		Brown, soft, sandy very clayey GRAVEL with high cobble content with high boulder content. Cobbles are 63mm to 200mm dia, angular, siltstone. Boulders are 200mm to 500mm dia, angular.	1
	3.00 - 3.40	В		2.80	85.83	× × × × × × × × × × × × × × × × × × ×	SILTSTONE. Recovered as cobbles & boulders of green siltstone.	•
				3.40	85.23	***** ***** ***** ***** *****	End of Pit at 3.400m	
ıbility:	Moderate.				10	Groundwa	Iter: None encountered.	

Plant: 13 Tonne Track Machine.

A<u>risings</u> Backfill:

Remarks: Trial pit terminated at 3.40m bgl due to rock.







Number:

TP03

Project Project No Engineer







Number:

TP03

Project Project No Engineer

_		Trial Pit No								
	oriority				021 4631 021 463		TP04			
g	eotechnical			www.prioritygeotechnical.ie						Sheet 1 of 1
Project	Longviou H	Dovolonment	Project No.			Co-ords: 5688	Date			
Name: Longview Housing Development				P19012			Level: 80.10	14/02/2019		
Locatio	n: Ballyvolane	Cork					Dimensions (m):			Scale
	Baily volaile					0			1:25	
Client:	Temporis Lt	td.					Depth: 3.40m BGL	2.0		Logged DMC
ter e & fill	Sampl	es & In S	Situ Testing	Depth	Level			0 4 4	.	
Water Strike & Backfill	문 전 Depth (m) Type Results		Results	(m) (m OD)		Legend	Stratum Description			
777287772						7////	(

Client:	Temporis L	td.					Depth: Cogged Logged DMC DMC
Water Strike & Backfill	Samp	les & In Situ	ı Testing	Depth	Level		
	Depth (m)	Туре	Results	(m)	(m OD)	Legend	·
				0.30	79.80		(TOPSOIL).
	0.50 - 2.00 0.50 - 2.00	B B					Red, soft, clayey very sandy GRAVEL with high cobble content. Gravel is fine to coarse, angular. Cobbles are 63mm to 150mm dia, angular to sub-angular, sandstone.
	2.50 - 3.40	В		2.40	77.70		SILTSTONE. Rock recovered as cobbles & boulders of green siltstone.
				3.40	76.70	××××× ×××××× ××××××	End of Pit at 3.400m
							4
ability:	Moderate. 13 Tonne Track				1	Groundwa	ater: None encountered.

Plant: 13 Tonne Track Machine.

Backfill: Arisings.

Remarks: Trial pit terminated 3.40m bgl due to rock.







Number:

TP04

Project Project No Engineer







Number:

TP04

Project Project No Engineer



Trial Pit No **TP05**

Sheet 1 of 1

Project Longview Housing Development Name:

Project No. P19012

Co-ords:568930E - 575025N

Date 14/02/2019 Scale

Location: Ballyvolane, Cork

<u>Arisings</u>

Remarks: Trial pit terminated at 2.90m bgl due to rock.

Backfill:

Dimensions (m):

Level: 81.67m OD

3.50

1:25 Logged

Client:	Temporis L	.td.					Depth: 2.90m BGL	Log	gged MC
Water Strike & Backfill		oles & In Sit		Depth	Level	Legend		, ,	
Stri	Depth (m)	Туре	Results	(m)	(m OD)			011	
	0.50 - 1.00 0.50 - 1.00	B B		0.30	81.37		(TOPSOIL). Orange, brown, soft, slightly sandy sli CLAY. Sand is fine to coarse. Gravel i sub-angular.	ghtly gravelly s fine to coarse,	
	1.00 - 1.80 1.00 - 1.80	B B		1.00	80.67		Brown, purple, soft, slightly sandy gramedium cobble content. Cobbles are dia, angular to sub-angular, sandstond	63mm to 200mm	1 -
	2.00 - 2.90	В		1.80	79.87		COBBLES & BOULDERS. Rock reco tabular cobbles and boulders of greer siltstone/sandstone.	vered as angular /pink weathered	2 -
				2.90	78.77		End of Pit at 2.900m		3 -
									4 -
									5 -
	Moderate. 13 Tonne Track				1	Groundwa	ater: None encountered.		







Number:

TP05

Project Project No Engineer







Number:

TP05

Project Project No Engineer



Trial Pit No **TP06**

Sheet 1 of 1

Project Longview Housing Development Name:

Project No. P19012

Co-ords:568872E - 574999N **Level:** 78.00m OD

Date 14/02/2019

Location: Ballyvolane, Cork

Dimensions (m):

Scale 1:25

Backfill: Arisings.

Remarks: Trial pit terminated at 2.10m bgl due to rock.

Depth:

2.00 Logged

3.50

Temporis L	td.					2.10m BGI Logged	ı
Samp	les & In Situ	ı Testing	Depth	Level	Ţ <u>.</u>		
Depth (m)	Туре	Results	(m)	(m OD)	Legend		
0.40 - 0.90 0.40 - 0.90 0.40 - 0.90	B B D		0.40	77.60	5 -	Orange, brown, soft, gravelly SILT with high cobble content. Gravel is fine to coarse, angular to subangular. Cobbles are 63mm to 150mm dia, angular to sub-angular, sandstone.	
1.00 - 2.00	В		0.90	77.10	**************************************	SILTSTONE. Rock recovered as cobbles and boulders of green siltstone.	1 -
			2.10	75.90	××××× ××××× ××××××	End of Pit at 2.100m	2 -
							3 -
							4 -
							5 -
	Samp Depth (m) 0.40 - 0.90 0.40 - 0.90 0.40 - 0.90	0.40 - 0.90 B 0.40 - 0.90 B 0.40 - 0.90 D	Samples & In Situ Testing Depth (m) Type Results 0.40 - 0.90 B 0.40 - 0.90 B 0.40 - 0.90 D D	Depth (m) Type Results Depth (m)	Depth (m) Type Results Depth (m) Level (m OD)	Depth (m) Type Results Depth (m) Level (m OD)	Samples & In Situ Testing Depth (m) Type Results Depth (m) OD Type Results O.40 - 0.90 O.40 - 0.90 D O.40 - 0.90 D Orange, brown, soft, gravelly SILT with high cobble content. Gravel is fine to coarse, angular to sub-angular. Cobbles are 63mm to 150mm dia, angular to sub-angular, sandstone. O.90 T7.10 SILTSTONE. Rock recovered as cobbles and boulders of green siltstone.







Number:

TP06

Project Project No Engineer







Number:

TP06

Project Project No Engineer



Trial Pit No **TP07**

Sheet 1 of 1

Project Longview Housing Development Name:

Project No. P19012

Co-ords:568884E - 575142N **Level:** 79.64m OD

Date 14/02/2019 Scale

3.20 1:25

Backfill: Arisings

Remarks: Trial terminated at 3.50m bgl due to obstruction.

	Logged
	DMC

Samples & In Situ Testing Depth (m) Type Results Depth (m) Type Ty		: Ballyvolane	, COIK					Dimensions (m):	25
Samples & In Situ Testing Depth (m) Type Results Depth (m) Corange, brown, soft, slightly sandy slightly gravelly SILT Sand is fine to coarse. Gravel is fine to coarse, sub-angular.		Temporis L	td.					Depth: 3.50m BGL 1:2	ged IC
0.30 79.34 Orange, brown, soft, slightly sandy slightly gravelly SILT. Sand is fine to coarse. Gravel is fine to coarse, sub-angular. Red, soft to firm, clayey sandy GRAVEL with high cobble content. Gravel is fine to coarse. Cobbles are 63mm to 120mm dia, angular to sub-angular, siltstone. 1.90 - 2.50 B 1.90 77.74 SILTSTONE. Rock recovered as angular pink siltstone gravel, cobbles & boulders. Boulder size increasing with depth.	kfill	Samp	les & In Situ	ı Testing	Depth		Logond		
0.80 - 1.80 B B 1.90 77.74 1.90 - 2.50 B 1.90 - 2.50 B 1.90 - 3.50 B B 1.90 77.74 3.00 - 3.50 B 1.90 77.74	Вас	Depth (m)	Туре	Results	(m)	(m OD)	Legena		
0.80 - 1.80 B 0.80 - 1.80 B 1.90 77.74 1.90 - 2.50 B 1.90 - 2.50 B 1.90 - 3.50 B 1.90 T.8.40 B 1.90								(TOPSOIL)	
0.80 - 1.80 0.80 - 1.80 B 0.80 - 1.80 B 1.90 - 2.50 B 1.90 - 2.50 B 3.00 - 3.50 B 1.90 - 3.50 B					0.30	79.34	* * * * * * * * * * * * * * * * * * *	SILT. Sand is fine to coarse. Gravel is fine to coarse,	
1.90 - 2.50 B X			B B		0.75	78.89	*****	cobble content. Gravel is fine to coarse. Cobbles are	
1.90 - 2.50 B X									
		1.90 - 2.50 1.90 - 2.50	B B		1.90	77.74	X X X X X X X X X X X X X X X X X X X	gravel, cobbles & boulders. Boulder size increasing	:
End of Pit at 3.500m		3.00 - 3.50	В		3 50	76 14	X X X X X X X X X X X X X X X X X X X		
					0.00	70.11		End of Pit at 3.500m	
									,







Number:

TP07

Project Project No Engineer







Number:

TP07

Project Project No Engineer



Trial Pit No **TP08**

Project
Name:

Longview Housing Development

Project No. P19012

Co-ords:568761E - 575006N **Level:** 69.70m OD Date 14/02/2019 Scale

Sheet 1 of 1

Location: Ballyvolane, Cork

Dimensions (m):

3.00

1:25 Logged

Client:	Temporis L	td.					Depth: 2 Logg	ed
_ % ≣		les & In Sit	u Testing	Depth	Level		1.80m BGL DM0	<u>, </u>
Water Strike & Backfill	Depth (m)	Туре	Results	(m)	(m OD)	Legend	Stratum Description	
	0.50 - 1.00 0.50 - 1.00	B B		0.30	69.40		(TOPSOIL) Brown, purple, slightly sandy gravelly SILT with medium cobble content. Sand is fine to coarse. Gravel is fine to coarse, sub-angular. Cobbles are 63mm to 150mm dia, angular to sub-angular.	-
	1.20 - 1.80	В		1.10	68.60		SANDSTONE. Rock recovered as blocky cobbles and boulders of sandstone.	1 -
				1.80	67.90			-
				1.60	67.90		End of Pit at 1.800m	2 -
								-
								3 -
								-
								4 -
								-
Stability:							ater: Slow flow 1.8m	5 -

Stability: Moderate.

Plant: 13 Tonne Track Machine.

Backfill: Arisings.

Groundwater: Slow flow 1.8m..

Remarks: Trial pit terminated at 1.80m bgl due to rock.







Number:

TP08

Project Project No Engineer







Number:

TP08

Project Project No Engineer



Trial Pit No **TP09**

Sheet 1 of 1

Project No. Project Longview Housing Development Name: P19012

Co-ords:568854E - 575165N

Level: 76.43m OD

14/02/2019 Scale

Date

Location: Ballyvolane, Cork Client: Temporis Ltd.

Dimensions (m):

1:25

2.00 Depth: Logged

3.00

ent:	Temporis L	.td.					3.20m BGL DMC
e ∰ Ø	Samp	les & In Sit	tu Testing	Depth	Level		
Strike & Backfill	Depth (m)	Туре	Results	(m)	(m OD)	Legend	Stratum Description
				0.30	76.13		(TOPSOIL)
	0.50 - 1.00 0.50 - 1.00	B D		0.00			Purple, brown, soft, slightly sandy gravelly CLAY. Sand is fine to coarse.
	1.00 - 1.70 1.00 - 1.70 1.00 - 1.70	B B D		1.00	75.43		Purple, brown, soft, slightly sandy gravelly CLAY with medium cobble content. Sand is fine to coarse. Gravel is fine to coarse, sub-angular. Cobbles are 63mm to 120mm dia, sub-angular, sandstone.
	2.00 - 2.80 2.00 - 2.80 2.00 - 2.80	B B D		1.70	74.73		Cream, mottled black, firm, slightly sandy slightly gravelly CLAY. Sand is fine to coarse. Gravel is fine to coarse, sub-angular.
	2.90 - 3.20	В		2.90	73.53		SILTSTONE. Rock recovered as angular pink gravel
						× × × × × × × × × × × × × × × × × × ×	and cobbles of siltstone.
				3.20	73.23	*****	End of Pit at 3.200m
	Moderate.						I ter: Trickling flow 1.8m.

Plant: 13 Tonne Track Machine.

A<u>risings</u> Backfill:

Remarks: Trial pit terminated at 3.20m bgl due to obstruction.







Number:

TP09

Project Project No Engineer



Trial Pit No **TP10**

Sheet 1 of 1

Project No. Project Longview Housing Development Name:

Co-ords:568808E - 575193N P19012 **Level:** 71.02m OD

Date 14/02/2019

Dimensions (m):

Scale 1:25

3.30

ocation	ո։ Ballyvolane	e, Cork					Dimensions (m):	1:25
lient:	Temporis L	.td.					Depth: 3.60m BGL	Logged DMC
Strike & Backfill	Samp	les & In Situ	ı Testing	Depth	Level	Legend		
Stri	Depth (m)	Туре	Results	(m)	(m OD)	Legena		
	0.50 - 1.50 0.50 - 1.50 0.50 - 1.50	B B D		0.40	70.62		(TOPSOIL) Brown, soft, slightly sandy slightly slightly g Sand is fine to coarse. Gravel is fine to coar	ravelly SILT. se, sub-
				1.60	69.42		Brown, purple, soft, gravelly sandy SILT. G to coarse, angular to sub-angular.	ravel is fine
	2.00 - 2.70 2.00 - 2.70 2.00 - 2.70	B B D					to coarse, angular to sub-angular.	2
	2.80 - 3.60	В		2.70	68.32	X	SILTSTONE. Recorded as pink angular gra cobbles.	vel and
				3.60	67.42	X X X X X X X X X X X X X X X X X X X	End of Pit at 3.600m	
								2
oility:	Moderate.				l le	Groundwa	ater: Slow flow 3.6m.	

Backfill: Arisings.

Remarks: Trial pit terminated at 3.60m bgl due to rock.



Backfill: Arisings

Remarks: Trial pit terminated at 3.00m bgl due to rock.

Priority Geotechnical Ltd. Tel: 021 4631600 Fax: 021 4638690 www.prioritygeotechnical.ie Trial Pit No **TP11**

Sheet 1 of 1

Co-ords:569022E - 575247N Project No. Project Longview Housing Development Name: P19012

Level: 98.34m OD

Date 15/02/2019 Scale

Location: Ballyvolane, Cork

Dimensions (m):

2.50

3.50

1:25 Logged

t:	Temporis L	td.					Depth: 3.00m BGL 1:20 Logg	jed C
 ■	Samp	les & In Situ	ı Testing	Depth	Level	Ī		<u> </u>
Васк	Depth (m)	Туре	Results	(m)	(m OD)	Legend	Stratum Description	
	0.40 - 0.90 0.40 - 0.90 0.40 - 0.90	B B D		0.40	97.94		(TOPSOIL) Orange, brown, soft, slightly sandy slightly gravelly SILT. Sand is fine to coarse. Gravel is fine to medium.	
	1.00 - 3.00	В		0.90	97.44		COBBLES & BOULDERS. Angular blocks of sandstone.	2 -
				3.00	95.34		End of Pit at 3.000m	4 -
								5







Number:

TP11

Project Project No Engineer







Number:

TP11

Project Project No Engineer



Trial Pit No **TP12**

Sheet 1 of 1

Project No. Project Longview Housing Development Name: P19012

Co-ords:569036E - 575319N

Level: 98.68m OD

Date 15/02/2019 Scale

Location: Ballyvolane, Cork

A<u>risings</u>

Remarks: Trial pit terminated at 2.40m due to rock.

Backfill:

Dimensions (m):

3.00

1:25 Logged

Client:	Temporis L	₋td.					Depth: 2.40m BGL	Logg DM	jed C
Water Strike & Backfill		oles & In Situ		Depth	Level	Legend		n	
St.	Depth (m)	Туре	Results	(m)	(m OD)		(TOPSOIL)		
	0.50 - 1.50 0.50 - 1.50	B B		0.30	98.38		Brown, soft, sandy very silty GRAVEL w content with high boulder content. Grave coarse, sub-angular. Cobbles & boulder sandstone.	el is fine to	1
	2.00 - 2.40 2.00 - 2.40	ВВВ		1.50	97.18		very clayey very sandy GRAVEL clayey cobble content and Boulder content Cob boulders are angular green Siltstone (W rockmass).	bles &	2
	2.30 2.10			2.40	96.28		End of Pit at 2.400m		
									3
									2
									5
tability.	Moderate.					Groundwa	ater: Slow flow1.5m.		







Number:

TP12

Project Project No Engineer







Number: TP12

Project Project No Engineer



Trial Pit No **TP13**

Sheet 1 of 1

Co-ords:568917E - 575422N Project No. Project Longview Housing Development Name: P19012

Level: 80.10m OD

Date 15/02/2019

Location: Ballyvolane, Cork

Dimensions (m):

Scale 1:25

Depth: Client: Temporis Ltd.

Logged

3.50

lient:	Temporis L	.lu.					2.50m BGL DMC
Strike & Backfill		les & In Sit	-	Depth	Level	Legend	Stratum Description
Stri	Depth (m)	Туре	Results	(m)	(m OD)	V////////	(TOPSOIL)
	0.50 - 1.50 0.50 - 1.50 0.50 - 1.50	B B B		0.40	79.70		Brown, soft, slightly sandy gravelly CLAY with high cobble content. Sand is fine to coarse. Gravel is fine to coarse, sub-angular. Cobbles are 63mm to 120mm dia, angular.
				1.60	78.50		ROCK Recovered as pink tabular angular cobbles &
	2.00 - 2.50 2.00 - 2.50	B B				X X X X X X X X X X X X X X X X X X X	ROCK. Recovered as pink tabular angular cobbles & boulders of siltstone dipping sub-vertical.
				2.50	77.60	* * * * * * * * * * * * * * * * * * *	End of Pit at 2.500m
	Moderate.						ter: None encountered.

Plant: 13 Tonne Track Machine.

A<u>risings</u> Backfill:

Remarks: Trial pit terminated at 2.50m bgl due to obstruction.







Number:

TP13

Project Project No Engineer





Number: TP13 Project Longview, Ballyhooly Rd Cork Project No P19012 Engineer MHL Consulting Engineers



Trial Pit No **TP14**

Sheet 1 of 1

Project Longview Housing Development Name:

Project No. P19012

Co-ords:569030E - 575394N **Level:** 94.29m OD

Date 15/02/2019

Scale 1:25

3.00

cation	: Ballyvolane	e, Cork					Dimensions (m):	1:25
ent:	Temporis L	td.					Depth: 2.50m BGL	ogged DMC
Strike & Backfill	Samp Depth (m)	les & In Situ	u Testing Results	Depth (m)	Level (m OD)	Legend	Stratum Description	
у ш	0.50 - 1.50 0.50 - 1.50 0.50 - 1.50	B B B	rocale	0.40	93.89		(TOPSOIL) Brown, clayey sandy GRAVEL with high cobble content. Cobbles are 63mm to 200mm dia, angular to sub-angular, siltstone.	
	0.50 - 1.50							
	1.80 - 2.50	В		1.60	92.69		COBBLES & BOULDERS. Recovered as pink angular tabular cobbles & boulders, 200mm to 400mm dia.	
				2.50	91.79	0.00	End of Pit at 2.500m	
	Moderate.						ater: None encountered.	

A<u>risings</u> Backfill:

Remarks: Trial pit terminated at 2.50m bgl due to obstruction.







Number:

TP14

Project Project No Engineer







Number:

TP14

Project Project No Engineer



Project

Name:

Priority Geotechnical Ltd. Tel: 021 4631600 Fax: 021 4638690 www.prioritygeotechnical.ie

Trial Pit No **TP15** Sheet 1 of 1

Co-ords:569114E - 575398N Project No. Longview Housing Development

P19012

Level: 103.60m OD

Date 15/02/2019

Dimensions (m):

Scale 1:25

3.20

: Ballyvolane	-, COIK					Dimensions (m):	
Temporis L	td.					Logg 2.90m BGL DM	jed C
Samp	les & In Situ	ı Testing	Depth	Level	Legend		
Depth (m)	Туре	Results	(m)	(m OD)	Legena		Ţ,
0.50 - 1.70 0.50 - 1.70	B B		0.40	103.20		Brown, soft, slightly sandy gravelly SILT with high cobble content. Gravel is fine to coarse, sub-angular. Cobbles are angular to sub-angular, siltstone.	
2.00 - 2.90 2.00 - 2.90	B B		1.75	101.86		COBBLES. Recovered as pink angular tabular cobbles of Mudstone/Siltstone.	
			2.90	100.70		End of Pit at 2.900m	
	Temporis L' Sampl Depth (m) 0.50 - 1.70 0.50 - 1.70	Temporis Ltd. Samples & In Situ Depth (m) Type 0.50 - 1.70 B 0.50 - 1.70 B 2.00 - 2.90 B	Temporis Ltd. Samples & In Situ Testing Depth (m) Type Results 0.50 - 1.70 B B 0.50 - 1.70 B	Depth (m) Type Results Depth (m) 0.50 - 1.70 B 0.50 - 1.70 B 2.00 - 2.90 B 2.00 - 2.90 B B	Temporis Ltd. Samples & In Situ Testing Depth (m) Level (m OD)	Samples & In Situ Testing Depth (m) Level (m OD)	Temporis Ltd. Samples & In Situ Testing Depth (m) Type Results 0.40 103.20 Brown, soft, slightly sandy gravelly SiLT with high cobble content. Gravel is fine to coarse, sub-angular. Cobbles are angular to sub-angular, siltstone. 1.75 101.86 COBBLES. Recovered as pink angular tabular cobbles of Mudstone/Siltstone.

13 Tonne Track Machine. Plant:

Backfill: Arisings

Remarks: Trial pit terminated at 2.90m bgl due to rock.







Number:

TP15

Project Project No Engineer







Number:

TP15

Project Project No Engineer



Trial Pit No **TP16**

Sheet 1 of 1

Project Longview Housing Development Name:

Project No. P19012

Co-ords:569563E - 575058N **Level:** 127.27m OD

Date 15/02/2019

Location: Ballyvolane, Cork

Dimensions (m):

Scale 1:25

Client: Temporis Ltd. Depth:

3.00

Logged

Client:	Temporis Lt	td.					2.40m BGL DMC
Water Strike & Backfill	Sampl	Depth	Level	Lamand			
Strijk Bac	Depth (m)	Туре	Results	(m)	(m OD)	Legend	·
				0.40	126.87		Purple, brown, soft, slightly gravelly SILT. Gravel is fine to coarse, sub-angular.
	1.00 - 2.00 1.00 - 2.00	B B		0.90	126.37		Brown, soft, very gravelly sandy SILT with high cobble content. Gravel is fine to coarse, sub-angular. Cobbles are 63mm to 200m dia, angular to sub-angular.
	2.00 - 2.40	В		2.00	125.27		COBBLES & BOULDERS. Recovered as angular blocks of brown/green cobbles & boulders of sandstone.
							4
ability:	Good.				1	Groundwa	ater: Slow flow 2.4m.

Plant: 13 Tonne Track Machine.

<u>Arisings</u> Backfill:

Remarks: Trial pit terminated at 2.40m bgl due to rock.







Number:

TP16

Project Project No Engineer







Number:

TP16

Project Project No Engineer

Priority Geotechnical Ltd. priority geotechnical Tel: 021 4631600 Fax: 021 4638690 www.prioritygeotechnical.ie Co-ords:569676E - 574955N Project No. Project Longview Housing Development Name: P19012 **Level:** 127.70m OD 3.00 Dimensions (m): Location: Ballyvolane, Cork 2.00

Trial Pit No

TP17

Sheet 1 of 1

Date

15/02/2019

Scale

1:25

t:	Temporis L	td.			Depth: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		Logged DMC		
	Samples & In Situ Testing Depth Level (m) (m OD) Legen								
Strike & Backfill	Depth (m)	Туре	Results	(m)	(m OD)	(m OD)	(TOPSOIL)		
				0.40	127.30		Brown, clayey sandy GRAVEL. Gravel is fi	ne to coarse.	
	1.00 - 2.00 1.00 - 2.00	B B		1.00	126.70		Brown, slightly sandy gravelly CLAY with I content. Cobbles are 63mm to 200mm dia siltstone.	nigh cobble angular,	1
									2
	2.30 - 2.70	В		2.30	125.40	0 2 0 0	COBBLES. Recovered as angular cobbles	of siltstone.	
				2.70	125.00		End of Pit at 2.700m		

A<u>risings</u> Backfill:

Remarks: Trial pit terminated at 2.70m bgl due to rock.







Number:

TP17

Project Project No Engineer







Number:

TP17

Project Project No Engineer

pgl _{pe}	riority otechnical				Tel: Fax:	021 463 021 463		Trial Pit TP1 Sheet 1	8
Project Name:	Longview H	lousing D	evelopment		ect No.		Co-ords: 569586E - 575249N	Date	е
	: Ballyvolane	e, Cork		P190	12		Level: 126.65m OD Dimensions (m): 3.50	15/02/2 Scale	е
Client:	·					Depth: 8 2.90m BGL	Logge	Logged DMC	
Water Strike & Backfill	Samples & In Situ Testing Depth Depth (m)				Level	Legend			
Stri W	Depth (m)	Туре	Results	(m)	(m OD)		(TOPSOIL)		
	0.50 - 1.50 2.00 - 2.90	В		0.40	126.25		COBBLES & BOULDERS. Rock recovered & boulders.	as cobbles	1 -

2.90

123.75

Groundwater: None encountered.

End of Pit at 2.900m

3

4

5 -

Stability: Moderate.
Plant: 13 Tonne Track Machine.
Backfill: Arisings.
Remarks: Trial pit terminated at 2.90m bgl due to competent rock.







Number:

TP18

Project Project No Engineer







Number: TP18 Project Project No Engineer Longview, Ballyhooly Rd Cork P19012 MHL Consulting Engineers



Priority Geotechnical Ltd. Tel: 021 4631600 Fax: 021 4638690 www.prioritygeotechnical.ie Trial Pit No **TP19**

Sheet 1 of 1

Project No. Project Longview Housing Development Name: P19012

Co-ords:569512E - 574922N **Level:** 127.70m OD

Date 19/02/2019

Dimensions (m):

Scale 1:25

2.50

ocation	ı: Ballyvolane	e, Cork					Dimensions (m):				
lient:	Temporis L	td.					Depth: 1.90 m BGL □	Logged AO			
Strike & Backfill	Samp	les & In Situ	ı Testing	Depth	Level	Legend		-			
Stri	Depth (m)	epth (m) Type Results		(m)	(m OD)		(TOPSOIL). Brown, clayey slightly gravelly SAND whigh cobble content. Sand is fine to coarse. Gravel fine to coarse, angular, siltstone. Cobbles are angul siltstone.	vith is lar,			
	0.60 - 1.00	В		0.60	127.10		Brown, clayey sandy GRAVEL with high cobble content. Gravel is fine to coarse, angular. Cobbles are angular, siltstone.				
	1.50 - 1.90	В		1.90	125.80		End of Pit at 1.900m	2			
								3			
								4			
ability:	Moderate.				I	Groundwa	ater: None encountered.	5			

Plant:

13 Tonne Track Machine

Backfill: Arisings

Remarks: Trial pit terminated 1.90m bgl due to bedrock.







Number:

TP19

Project Project No Engineer





Number: TP19 Project Longview, Ballyhooly Rd Cork Project No P19012 Engineer MHL Consulting Engineers



Priority Geotechnical Ltd. Tel: 021 4631600 Fax: 021 4638690 www.prioritygeotechnical.ie Trial Pit No **TP20**

Sheet 1 of 1

Project Longview Housing Development Name:

Project No. P19012

Co-ords:569446E - 575181N Level: 128.24m OD

Date 19/02/2019

Location: Ballyvolane, Cork

Dimensions (m):

Scale 1:25

Client: Temporis Ltd. Depth:

	1.20
	Logged AO

2.50

lient:	Temporis L	td.					2.50m BGL 2099ed AO
Strike & Backfill	Samp	les & In Si	tu Testing	Depth	Level	Legend	Stratum Description
Stri	Depth (m)	Туре	Results	(m)	(m OD)	Legena	
	0.50 - 1.00	is fine to coarse. G		Orange, brown, slightly silty, slightly sandy GRAVEL with high cobble content with low boulder content. Sand is fine to coarse. Gravel is angular, siltstone. Boulders are 200mm to 400mm dia, siltstone.			
	1.50 - 2.00	В					
				2.10	126.14		COBBLES & BOULDERS. Recovered as weathered angular siltstone bedrock of cobbles & boulders.
				2.50	125.74		End of Pit at 2.500m
							3
bilitv:	Moderate.					Groundwa	ater: None encountered.

Plant: 13 Tonne Track Machine.

Arisings Backfill:

Remarks: Trial pit terminated at 2.50m bgl due to bedrock.







Number:

TP20

Project Project No Engineer







Number:

TP20

Project Project No Engineer

pgl _{pi}	riority otechnical				Tel: Fax:	021 463 ⁴		Trial Pit TP2 Sheet 1	1	
Project	Longview I	Housing D	evelopment	1 -	ct No.		Co-ords :568758E - 575330N	Date		
Name:			•	P190	12		Level: 69.92m OD 3.00	19/02/2019 Scale		
Client:	: Ballyvolane Temporis L						Dimensions (m): Depth: 3.50m BGL	1:25 Logge AO	ogged	
. % ≡	Samples & In Situ Testing		ı Testing	Depth	Level	Ī		<u>AO</u>		
Water Strike & Backfill	Depth (m)	Туре	Results	(m)	(m OD)	Legend	Stratum Description			
	1.00 - 1.50	В		0.60	69.32		Purple, clayey very sandy GRAVEL with low cocontent. Gravel is fine to coarse, sub-angular to rounded, sandstone/siltstone. Cobbles are sub to rounded, sandstone/siltstone.)	1	
	2.00 - 2.50	В							2	

2.50 67.42 Purple, gravelly very clayey SAND with low cobble content. Sand is fine to medium. Gravel is fine to coarse, sub-angular to rounded, sandstone/siltstone. 3.00 - 3.50 В 3 3.50 66.42 End of Pit at 3.500m 4 5 -Stability: Poor.
Plant: 13 Tonne Track Machine.
Backfill: Arisings.
Remarks: Trial pit terminated at 3.50m bgl due to pit wall instability. Groundwater: Steady flow 0.8m.







Number:

TP21

Project Project No Engineer







Number:

TP21

Project Project No Engineer



Priority Geotechnical Ltd. Tel: 021 4631600 Fax: 021 4638690 www.prioritygeotechnical.ie Trial Pit No **TP22**

Sheet 1 of 1

Date

Project
Name: Longview Housing Development

Project No. P19012 **Co-ords:**568789E - 575278N **Level:** 69.38m OD

19/02/2019

Location: Ballyvolane, Cork

Dimensions (m):

Scale 1:25

Client: Temporis Ltd.

Depth:

1:25 Logged AO

3.50

lient:	Temporis L	.td.			3.60m BGL Logged			
Strike & Backfill	Samp	les & In Si	tu Testing	Depth	Level	Legend		
Strij	Depth (m)	Туре	Results	(m)	(m OD)	Legena	•	
	0.50 - 1.00	.50 - 1.00 B				69.08		(TOPSOIL). Brown, clayey, slightly gravelly SAND. Pale purple, clayey sandy GRAVEL with low cobble content. Gravel is fine to coarse, sub-angular to rounded, sandstone. Cobbles are sub-angular to rounded, sandstone.
				1.00	68.38		Purple, clayey very sandy GRAVEL. Gravel is fine to coarse, angular to sub-rounded, sandstone/siltstone.	
•	1.50 - 2.00	В		1.20	68.18		Red/brown, clayey, sandy GRAVEL with high cobble content. Sand is fine to coarse. Gravels are angular to sub-angular, sandstone/siltstone.	
				3.60	65.78		End of Pit at 3.600m	
bility:	Moderate. 13 Tonne Track				Į.	Groundwa	ater: Steady flow 2.5m.	

Plant: 13 Tonne Track Machine.

Backfill: Arisings.

Remarks: Trial pit terminated at 3.60m bgl due to bedrock.







Number:

TP22

Project Project No Engineer







Number:

TP22

Project Project No Engineer



Priority Geotechnical Ltd. Tel: 021 4631600 Fax: 021 4638690 www.prioritygeotechnical.ie Trial Pit No **TP23**

Sheet 1 of 1

Project Longview Housing Development Name:

Project No. P19012

Co-ords:568807E - 575376N **Level:** 70.97m OD

Date 19/02/2019

Location: Ballyvolane, Cork

Dimensions (m):

3.00 Scale 1:25

Client: Temporis Ltd.

2.00 Depth: 3.50m BGL

Logged

	Constant State Testing				3.50m BGL AO			
Backfill	Samp Depth (m)	les & In Si Type	tu Testing Results	Depth (m)	Level (m OD)	Legend	Stratum Description	
							(TOPSOIL). Brown, slightly organic slightly gravelly CLAY.	
				0.20	70.77	316. 3e	Orange, soft, slightly organic slightly sandy slightly gravelly CLAY.	
	0.50 - 1.00	В		0.40	70.57	NIE - 10 - 10 - 10 - 10 - 10 - 10 - 10 - 1	Grey, brown, soft, slightly organic slightly sandy slightly gravelly CLAY with low cobble content. Cobbles are sub-angular, sandstone/siltstone.	
	0.80	D		4.00	20.07			
				1.00	69.97		Purple, brown, clayey very sandy GRAVEL with high cobble content. Sand is fine to coarse. Gravel is fine to coarse, angular to sub-angular, sandstone/siltstone. Cobbles are sub-angular, sandstone/siltstone.	1
	1.50 - 2.00	В						2
								3
				3.50	67.47			
							End of Pit at 3.500m	
								4
	Poor.						ater: Steady flow 1.2m.	5

13 Tonne Track Machine. Plant:

Backfill: Arisings

Remarks: Trial pit terminated at 3.50m bgl due to pit wall instability.







Number:

TP23

Project Project No Engineer







Number:

TP23

Project Project No Engineer

pgl _p	riority otechnical				Fax:	021 463 021 463	1600 TD2	24	
Project	Longview F	Joueing D	evelopment	Proje	ct No.			Date	
Name:	Longview i	lousing D	evelopment	P190	12		Level: 70.87m OD 19/02/2		
Location	ı: Ballyvolane	e, Cork					Dimensions (m): 3.50 Scal		
Client:	Temporis L	.td.				Depth: 1:26 3.50m BGL AO	ed		
e e r Ge &	Samp	les & In Situ	ı Testing	Depth	Level				
Water Strike & Backfill	Depth (m) Type Results (m) (m OD) Legend Stratum Description					Stratum Description			
				0.40	70.47		(TOPSOIL). Brown, organic slightly sandy slightly gravelly CLAY.		
				0.60	70.27	316 34 X 7412 318 34	Light grey, soft, slightly organic slightly sandy slightly gravelly CLAY. Purple, brown, clayey very sandy GRAVEL with	_	
	1.00 - 1.50	В					medium cobble content. Cobbles are sub-angular, sandstone/siltstone.	1 -	
	2.50 - 3.00	В		2.20	68.67		Purple, brown, clayey gravelly SAND with low cobble content. Gravel is fine to coarse, sub-angular to sub-rounded, sandstone/siltstone. Cobbles are sub-angular to sub-rounded, sandstone/siltstone.	3 -	

3.50

67.37

Groundwater: Steady flow at several points between depths 0.7m and 2.2m.

5 -

End of Pit at 3.500m

Stability: Poor.
Plant: 13 Tonne Track Machine.
Backfill: Arisings.
Remarks: Trial pit terminated at 3.50m bgl due to pit wall instability.







Number:

TP24

Project Project No Engineer







Number:

TP24

Project Project No Engineer

KEY TO SYMBOLS - LABORATORY TEST RESULT

U Undisturbed Sample
P Piston Sample
TWS Thin Wall Sample
B Bulk Sample - Disturbed
D Jar Sample - Disturbed

W Water Sample pH Acidity/Alkalinity Index

SO₃ % - Total Sulphate Content (acid soluble)

SO₃ g/ltr - Water Soluble Sulphate (Water or 2:1 Aqueous Soil Extract)

+ Calcareous Reaction
Cl Chloride Content
Pl Plasticity Index

<425 % of material in sample passing 425 micron sieve

LL Liquid Limit
PL Plastic Limit
MC Water Content
NP Non Plastic
Yb Bulk Density
Yd Dry Density
Ps Particle Density

U/D Undrained/Drained Triaxial

U/C Unconsolidated/Consolidated Triaxial T/M Single Stage/Multistage Triaxial

100/38 Sample Diameter (mm)

REM Remoulded Triaxial Test Specimen

TST Triaxial Suction Test

V Vane Test

 $\begin{array}{ccc} \text{DSB} & \text{Drained Shear Box} \\ \text{RSB} & \text{Residual Shear Box} \\ \text{RS} & \text{Ring Shear} \\ \sigma_3 & \text{Cell Pressure} \\ \sigma_1\text{-}\sigma_3 & \text{Deviator Stress} \end{array}$

 σ_1 - σ_3 Deviator St c Cohesion

c_ Effective Cohesion Intercept

φ Angle of Shearing Resistance - Degrees
 φ Effective Angle of Shearing Resistance

εf Strain at Failure

* Failed under 1st Load

** Failed under 2nd Load

Untestable ## Excessive Strain

 $\begin{array}{lll} p_o & & \text{Effective Overburden Pressure} \\ m_v & & \text{Coefficient of Volume Decrease} \\ c_v & & \text{Coefficient of Consolidation} \end{array}$

Opt Optimum Nat Natural

Std Standard Compaction - 2.5kg Rammer (¶ CBR)
Hvy Heavy Compaction - 4.5kg Rammer (§ CBR)

Vib Vibratory Compaction
CBR California Bearing Ratio
Sat m.c. Saturation Moisture Content
MCV Moisture Condition Value



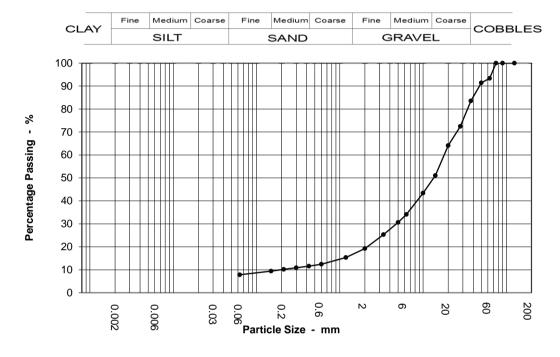
pgl priority	Natural Moisture Content/Atterberg Limits Summary BS 1377 : Part 2 : 1990 : Clause 3	Job Ref
Location	Longview Developments	P19012

Hole ID	Sample Ref	Depth (m)	Sample Type	Sample Description	МС	LL	PL	PI	% Pass 425
TP01	1	0.5	В	Silty sandy GRAVEL with low cobble content		46	33	13	31
TP01	2	0.5	D	Silty sandy GRAVEL	13				
TP02	2	0.5	D	Very sitly very sandy GRAVEL	33				
TP02	4	1.5	D	Slightly sandy gravelly SILT	16				
TP03	1	0.5	В	Very silty very sandy GRAVEL		42	30	12	54.4
TP03	3	0.5	D	Very silty very sandy GRAVEL	9				
TP03	4	1.5	В	Sandy very clayey GRAVEL		35	21	14	53.6
TP03	6	1.5	D	Sandy very clayey GRAVEL	12				
TP04	1	0.5	В	Clayey very sandy GRAVEL		30	22	8	39.5
TP05	1	0.5	В	Slightly sandy slightly gravelly CLAY	17	23	17	6	62.5
TP05	3	1	В	Slightly sandy gravelly CLAY	11	22	13	9	55.9
TP07	1	0.8	В	Clayey sandy GRAVEL		36	21	15	32.6
TP09	1	0.5	В	Very clayey very sandy GRAVEL		24	16	8	64.5
TP09	2	0.5	D	Very clayey very sandy GRAVEL	14				
TP09	3	1	В	Very clayey very sandy GRAVEL with low cobble content		24	15	9	58.7
TP09	5	1	D	Very clayey very sandy GRAVEL	13				
TP09	6	2	В	Slightly sandy slightly gravelly CLAY		29	16	13	77.4
TP12	1	0.5	В	Sandy very silty GRAVEL with medium cobble content		56	36	20	58.6
TP12	3	2	В	Very clayey very sandy GRAVEL with low cobble content		31	19	12	57.4
TP13	1	0.5	В	Clayey sandy GRAVEL with low cobble content		30	19	11	51.3

pgl priority	Natural Moisture Content/Atterberg Limits Summary BS 1377 : Part 2 : 1990 : Clause 3	Job Ref
Location	Longview Developments	P19012

Hole ID	Sample Ref	Depth (m)	Sample Type	Sample Description	МС	LL	PL	PI	% Pass 425
TP17	1	1	В	Clayey sandy GRAVEL with high cobble content		29	17	12	49.8

pgl _{priority}	PARTICLE SIZE DISTRIBUTION	Job Ref	P19012	
geotechnical	BS 1377 : Part 2 : 1990 : Clause 9	Borehole / Pit No	TP01	
Location	Longview Developments	Sample No	1	
		Depth	0.50 m	
Soil Description	Silty sandy GRAVEL with low cobble content	Sample type	В	



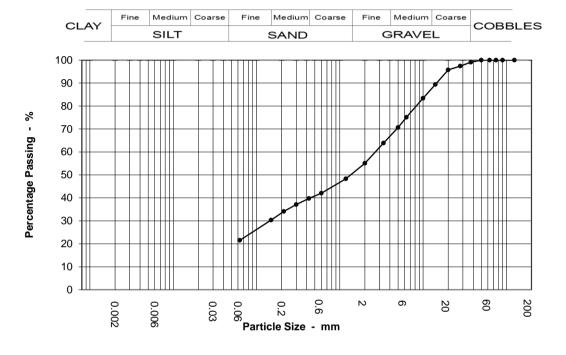
Sievir	Sieving		tation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	93		
50	91		
37.5	84		
28	72		
20	64		
14	51		
10	43		
6.3	34		
5	31		
3.35	25		
2	19		
1.18	15		
0.6	12		
0.425	12		
0.3	11		
0.212	10		
0.15	9		
0.063	8		

Test Method		
BS 1377 : Part 2 : 1990		
Sieving	Clause 9.3	
Sedimentation	N/A	

Sample Proportions			
Cobbles	7.0		
Gravel	74.0		
Sand	11.0		
Silt & Clay	8.0		

Grading Analysis			
D100	75.00		
D60	17.90		
D10	0.20		
Uniformity Coefficient 91.00			

pglpriority	PARTICLE SIZE DISTRIBUTION	Job Ref	P19012	
geotechnical	BS 1377 : Part 2 : 1990 : Clause 9	Borehole / Pit No	TP02	
Location	Longview Developments	Sample No	1	
		Depth	0.50 m	
Soil Description	cription Very sitly very sandy GRAVEL		В	



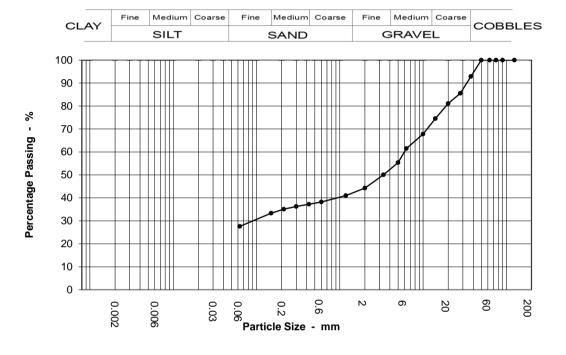
Sievir	Sieving		tation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	100		
37.5	99		
28	97		
20	96		
14	89		
10	83		
6.3	75		
5	71		
3.35	64		
2	55		
1.18	48		
0.6	42		
0.425	40		
0.3	37		
0.212	34		
0.15	30		
0.063	22		

Test Method		
BS 1377 : Part 2 : 1990		
Sieving	Clause 9.3	
Sedimentation	N/A	

Sample Proportions			
Cobbles	0.0		
Gravel	45.0		
Sand	34.0		
Silt & Clay	22.0		

Grading Analysis			
D100	50.00		
D60	2.66		
D10			
Uniformity Coefficient			

pglpriority	PARTICLE SIZE DISTRIBUTION	Job Ref	P19012	
geotechnical	BS 1377 : Part 2 : 1990 : Clause 9	Borehole / Pit No	TP02	
Location	Longview Developments	Sample No	3	
		Depth	1.50 m	
Soil Description	Slightly sandy gravelly SILT	Sample type	В	



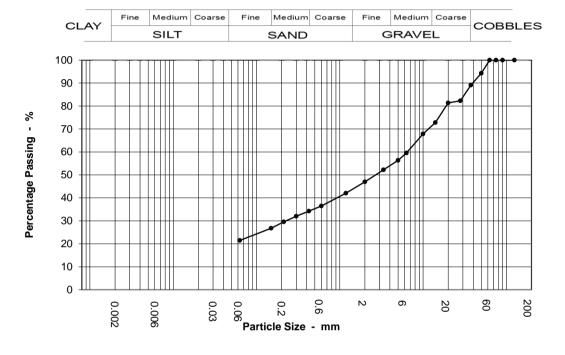
Sievir	Sieving		tation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	100		
37.5	93		
28	86		
20	81		
14	75		
10	68		
6.3	62		
5	55		
3.35	50		
2	44		
1.18	41		
0.6	38		
0.425	37		
0.3	36		
0.212	35		
0.15	33		
0.063	28		

Test Method	
BS 1377 : Part 2 : 1990	
Sieving	Clause 9.3
Sedimentation	N/A

Sample Proportions		
Cobbles	0.0	
Gravel	56.0	
Sand	17.0	
Silt & Clay	28.0	

Grading Analysis		
D100	50.00	
D60	5.95	
D10		
Uniformity Coefficient		

pgl _{priority}	PARTICLE SIZE DISTRIBUTION	Job Ref P19012	
geotechnical	BS 1377 : Part 2 : 1990 : Clause 9	Borehole / Pit No	TP03
Location	Longview Developments	Sample No	1
		Depth	0.50 m
Soil Description	Very silty very sandy GRAVEL	Sample type	В



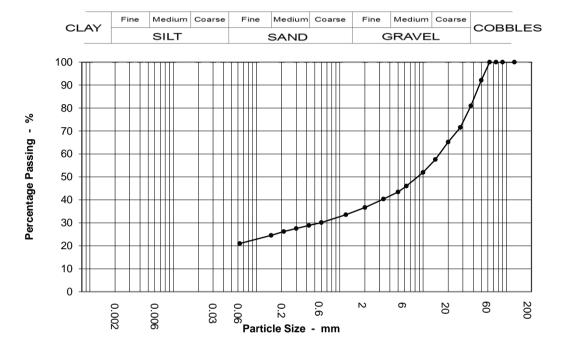
Sievir	ng	Sedimen	tation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	94		
37.5	89		
28	82		
20	81		
14	73		
10	68		
6.3	60		
5	56		
3.35	52		
2	47		
1.18	42		
0.6	36		
0.425	34		
0.3	32		
0.212	30		
0.15	27		
0.063	21		

Test Method	
BS 1377 : Part 2 : 1990	
Sieving	Clause 9.3
Sedimentation	N/A

Sample Proportions		
Cobbles	0.0	
Gravel	53.0	
Sand	26.0	
Silt & Clay	21.0	

Grading Analysis		
D100	63.00	
D60	6.46	
D10		
Uniformity Coefficient		

pgl _{priority}	PARTICLE SIZE DISTRIBUTION	Job Ref	P19012
geotechnical	BS 1377 : Part 2 : 1990 : Clause 9	Borehole / Pit No	TP03
Location	Longview Developments	Sample No	4
		Depth	1.50 m
Soil Description	Sandy very clayey GRAVEL	Sample type	В



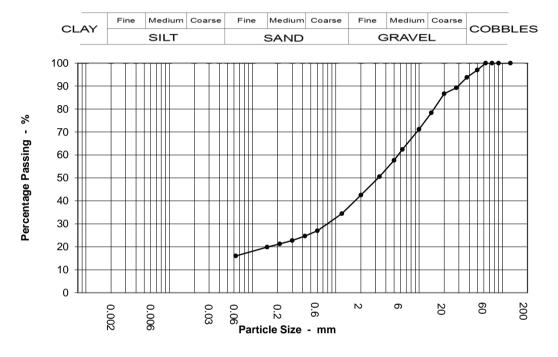
Sievii	ng	Sedimen	tation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	92		
37.5	81		
28	72		
20	65		
14	58		
10	52		
6.3	46		
5	43		
3.35	40		
2	37		
1.18	34		
0.6	30		
0.425	29		
0.3	28		
0.212	26		
0.15	25		
0.063	21		

Test Method	
BS 1377 : Part 2 : 1990	
Sieving	Clause 9.3
Sedimentation	N/A

Sample Proportions		
Cobbles 0.0		
Gravel	63.0	
Sand	16.0	
Silt & Clay	21.0	

Grading Analysis		
D100	63.00	
D60	15.70	
D10		
Uniformity Coefficient		

pglpriority	PARTICLE SIZE DISTRIBUTION	Job Ref	P19012
geotechnical	BS 1377 : Part 2 : 1990 : Clause 9	Borehole / Pit No	TP04
Location	Longview Developments	Sample No	1
		Depth	0.50 m
Soil Description	Clayey very sandy GRAVEL	Sample type	В



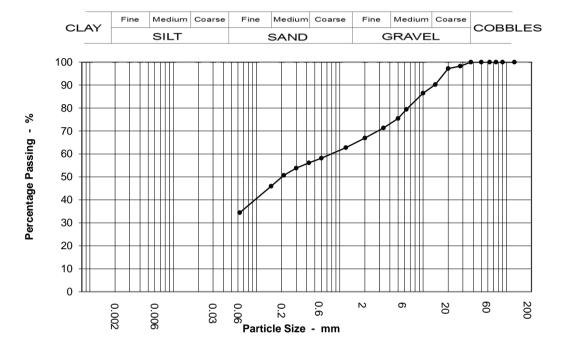
Sievir	Sieving		tation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	97		
37.5	94		
28	89		
20	87		
14	78		
10	71		
6.3	62		
5	58		
3.35	51		
2	43		
1.18	34		
0.6	27		
0.425	25		
0.3	23		
0.212	21		
0.15	20		
0.063	16		

Test Method	
BS 1377 : Part 2 : 1990	
Sieving Clause 9.3	
Sedimentation	N/A

Sample Proportions		
Cobbles	0.0	
Gravel	57.0	
Sand	27.0	
Silt & Clay	16.0	

Grading Analysis		
D100	63.00	
D60	5.60	
D10		
Uniformity Coefficient		

pgl _{priority}	PARTICLE SIZE DISTRIBUTION	Job Ref	P19012
geotechnical	BS 1377 : Part 2 : 1990 : Clause 9	Borehole / Pit No	TP05
Location	Longview Developments	Sample No	1
		Depth	0.50 m
Soil Description	Slightly sandy slightly gravelly CLAY	Sample type	В



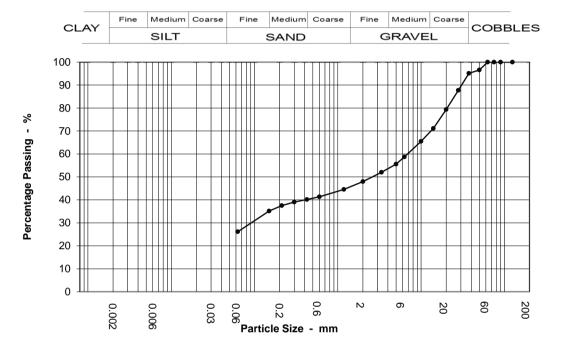
Sievii	ng	Sedimen	tation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	100		
37.5	100		
28	98		
20	97		
14	90		
10	86		
6.3	79		
5	75		
3.35	71		
2	67		
1.18	63		
0.6	58		
0.425	56		
0.3	54		
0.212	51		
0.15	46		
0.063	34		

Test Method	
BS 1377 : Part 2 : 1990	
Sieving Clause 9.3	
Sedimentation	N/A

Sample Proportions		
Cobbles	0.0	
Gravel	33.0	
Sand	32.0	
Silt & Clay	34.0	

Grading Analysis		
D100	37.50	
D60	0.79	
D10		
Uniformity Coefficient		

pglpriority	PARTICLE SIZE DISTRIBUTION	Job Ref	P19012
geotechnical	BS 1377 : Part 2 : 1990 : Clause 9	Borehole / Pit No	TP05
Location	Longview Developments	Sample No	3
		Depth	1.00 m
Soil Description	Slightly sandy gravelly CLAY	Sample type	В



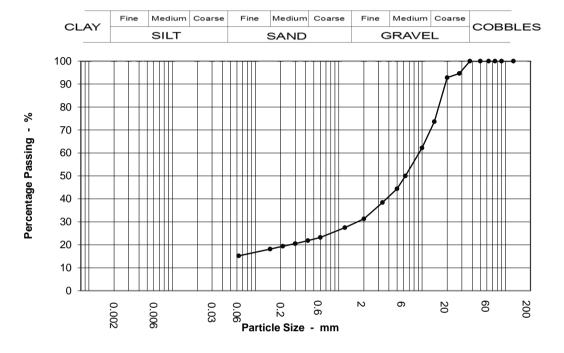
Sievir	ng	Sedimen	tation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	97		
37.5	95		
28	88		
20	79		
14	71		
10	66		
6.3	59		
5	56		
3.35	52		
2	48		
1.18	45		
0.6	41		
0.425	40		
0.3	39		
0.212	37		
0.15	35		
0.063	26		

Test Method		
BS 1377 : Part 2 : 1990		
Sieving	Clause 9.3	
Sedimentation	N/A	

Sample Proportions		
Cobbles	0.0	
Gravel	52.0	
Sand	22.0	
Silt & Clay	26.0	

Grading Analysis		
D100	63.00	
D60	6.87	
D10		
Uniformity Coefficient		

pglpriority	PARTICLE SIZE DISTRIBUTION	Job Ref	P19012
geotechnical	BS 1377 : Part 2 : 1990 : Clause 9	Borehole / Pit No	TP07
Location	Longview Developments	Sample No	1
		Depth	0.80 m
Soil Description	Clayey sandy GRAVEL	Sample type	В



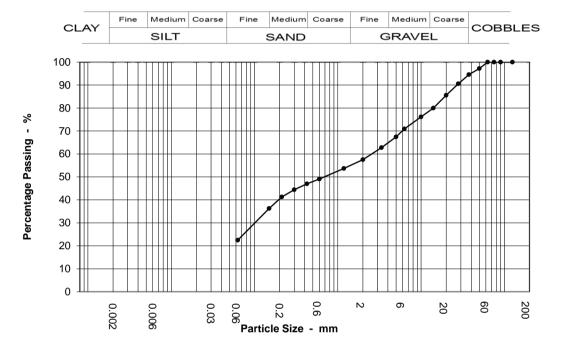
Sievii	ng	Sedimen	tation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	100		
37.5	100		
28	95		
20	93		
14	74		
10	62		
6.3	50		
5	44		
3.35	38		
2	31		
1.18	27		
0.6	23		
0.425	22		
0.3	21		
0.212	19		
0.15	18		
0.063	15		

Test Method		
BS 1377 : Part 2 : 1990		
Sieving	Clause 9.3	
Sedimentation	N/A	

Sample Proportions		
Cobbles	0.0	
Gravel	69.0	
Sand	16.0	
Silt & Clay	15.0	

Grading Analysis		
D100	37.50	
D60	9.21	
D10		
Uniformity Coefficient		

pglpriority	PARTICLE SIZE DISTRIBUTION	Job Ref	P19012
geotechnical	BS 1377 : Part 2 : 1990 : Clause 9	Borehole / Pit No	TP09
Location	Longview Developments	Sample No	1
		Depth	0.50 m
Soil Description	Very clayey very sandy GRAVEL	Sample type	В



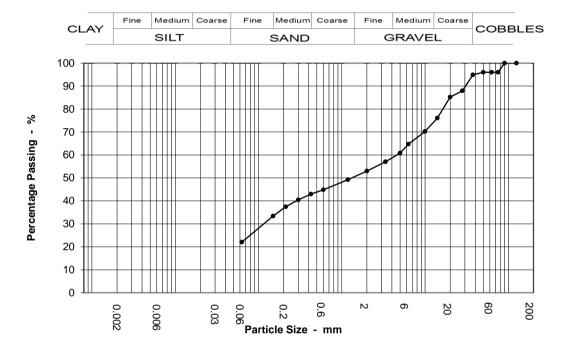
Sievir	ng	Sedimen	tation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	97		
37.5	95		
28	91		
20	86		
14	80		
10	76		
6.3	71		
5	67		
3.35	63		
2	58		
1.18	54		
0.6	49		
0.425	47		
0.3	44		
0.212	41		
0.15	36		
0.063	22		

Test Method		
BS 1377 : Part 2 : 1990		
Sieving	Clause 9.3	
Sedimentation	N/A	

Sample Proportions		
Cobbles	0.0	
Gravel	42.0	
Sand	35.0	
Silt & Clay	22.0	

Grading Analysis		
D100	63.00	
D60	2.55	
D10		
Uniformity Coefficient		

pgl _{priority}	PARTICLE SIZE DISTRIBUTION	Job Ref	P19012	
geotechnical	BS 1377 : Part 2 : 1990 : Clause 9	Borehole / Pit No	TP09	
Location	Longview Developments	Sample No	3	
		Depth	1.00 m	
Soil Description	Very clayey very sandy GRAVEL with low cobble content		В	



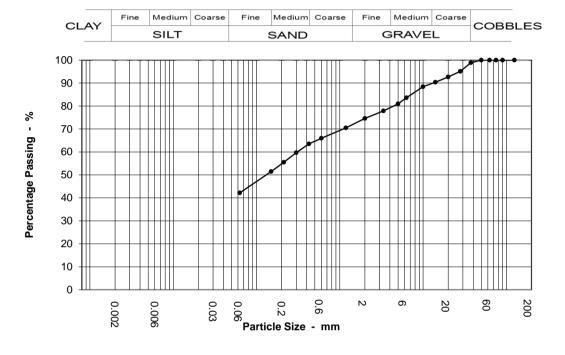
Sieving		Sedimen	tation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	96		
63	96		
50	96		
37.5	95		
28	88		
20	85		
14	76		
10	70		
6.3	65		
5	61		
3.35	57		
2	53		
1.18	49		
0.6	45		
0.425	43		
0.3	40		
0.212	37		
0.15	33		
0.063	22		

Test Method		
BS 1377 : Part 2 : 1990		
Sieving	Clause 9.3	
Sedimentation	N/A	

Sample Proportions			
Cobbles	4.0		
Gravel	43.0		
Sand	31.0		
Silt & Clay	22.0		

Grading Analysis			
D100	90.00		
D60	4.58		
D10			
Uniformity Coefficient			

pglpriority	PARTICLE SIZE DISTRIBUTION	Job Ref	P19012	
geotechnical	BS 1377 : Part 2 : 1990 : Clause 9	Borehole / Pit No	TP09	
Location	Longview Developments	Sample No	6	
		Depth	2.00 m	
Soil Description Slightly sandy slightly gravelly CLAY		Sample type	В	



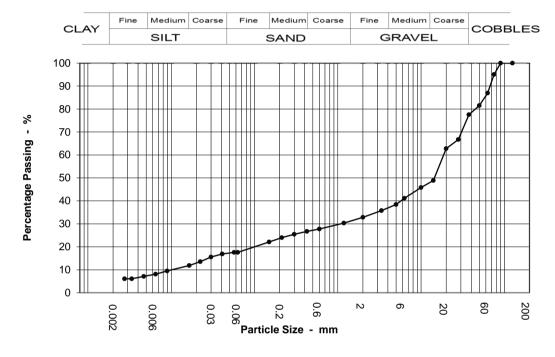
Sievir	Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing	
125	100			
90	100			
75	100			
63	100			
50	100			
37.5	99			
28	95			
20	93			
14	90			
10	88			
6.3	84			
5	81			
3.35	78			
2	75			
1.18	70			
0.6	66			
0.425	64			
0.3	60			
0.212	56			
0.15	51			
0.063	42			

Test Method		
BS 1377 : Part 2 : 1990		
Sieving	Clause 9.3	
Sedimentation	N/A	

Sample Proportions			
Cobbles	0.0		
Gravel	25.0		
Sand	32.0		
Silt & Clay	42.0		

Grading Analysis			
D100	50.00		
D60	0.31		
D10			
Uniformity Coefficient			

pgl _{priority}	PARTICLE SIZE DISTRIBUTION	Job Ref	P19012	
geotechnical	BS 1377 : Part 2 : 1990 : Clause 9	Borehole / Pit No	TP12	
Location	Longview Developments	Sample No	1	
		Depth	0.50 m	
Soil Description Sandy very silty GRAVEL with medium cobble content		Sample type	В	



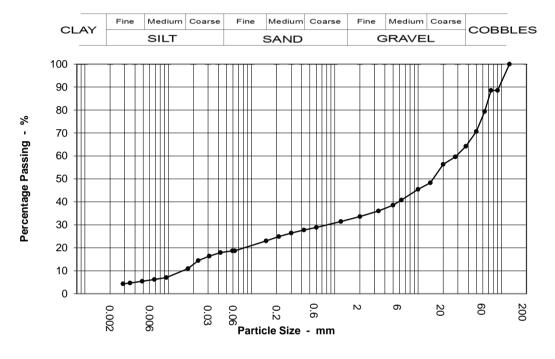
Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.057	18
90	100	0.041	17
75	95	0.030	16
63	87	0.022	14
50	82	0.016	12
37.5	78	0.009	9
28	67	0.006	8
20	63	0.005	7
14	49	0.003	6
10	46	0.003	6
6.3	41	0.001	5
5	38		
3.35	36		
2	33		
1.18	30		
0.6	28		
0.425	27		
0.3	25		
0.212	24		
0.15	22		
0.063	18		

Test Method	
BS 1377 : Part 2 : 1990	
Sieving	Clause 9.5
Sedimentation	Clause 9.5

Sample Proportions		
Cobbles	13.0	
Gravel	54.0	
Sand	15.0	
Silt	12.0	
Clay	5.0	

Grading Analysis		
D100	90.00	
D60	18.60	
D10	0.01	
Uniformity Coefficient 1800.00		

pgl _{priority}	PARTICLE SIZE DISTRIBUTION	Job Ref	P19012	
geotechnical	BS 1377 : Part 2 : 1990 : Clause 9	Borehole / Pit No	TP12	
Location	Longview Developments	Sample No	3	
		Depth	2.00 m	
Soil Description	Very clayey very sandy GRAVEL with low cobble content	Sample type	В	



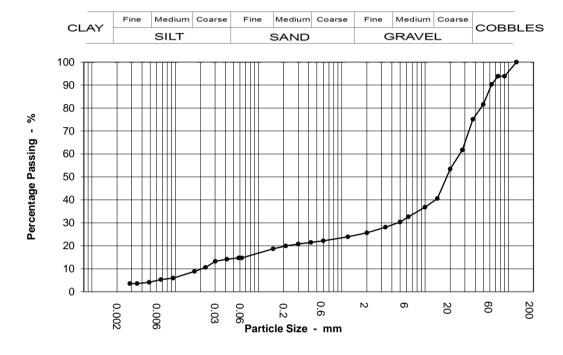
Sievii	ng	Sedimen	tation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.059	19
90	89	0.042	18
75	89	0.031	16
63	79	0.023	14
50	71	0.017	11
37.5	64	0.010	7
28	60	0.007	6
20	56	0.005	5
14	48	0.003	5
10	45	0.003	4
6.3	41	0.002	4
5	39		
3.35	36		
2	34		
1.18	31		
0.6	29		
0.425	28		
0.3	26		
0.212	25		
0.15	23		
0.063	19		

Test Method	
BS 1377 : Part 2 : 1990	
Sieving	Clause 9.5
Sedimentation	Clause 9.5

Sample Proportions		
Cobbles	21.0	
Gravel	46.0	
Sand	15.0	
Silt	15.0	
Clay	4.0	

Grading Analysis		
D100	125.00	
D60	28.60	
D10	0.02	
Uniformity Coefficient 1900.00		

pgl _{priority}	PARTICLE SIZE DISTRIBUTION	Job Ref	P19012
geotechnical	BS 1377 : Part 2 : 1990 : Clause 9	Borehole / Pit No	TP13
Location	Longview Developments	Sample No	1
		Depth	0.50 m
Soil Description	Clayey sandy GRAVEL with low cobble content	Sample type	В



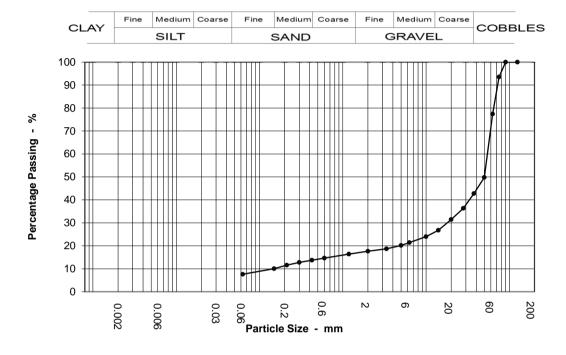
Sievii	ng	Sedimen	tation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.058	15
90	94	0.042	14
75	94	0.030	13
63	90	0.023	11
50	82	0.017	9
37.5	75	0.009	6
28	62	0.007	5
20	53	0.005	4
14	41	0.003	4
10	37	0.003	4
6.3	33	0.002	3
5	30		
3.35	28		
2	26		
1.18	24		
0.6	22		
0.425	22		
0.3	21		
0.212	20		
0.15	19		
0.063	15		

Test Method		
BS 1377 : Part 2 : 1990		
Sieving	Clause 9.5	
Sedimentation	Clause 9.5	

Sample Proportions		
Cobbles	10.0	
Gravel	65.0	
Sand	11.0	
Silt	12.0	
Clay	3.0	

Grading Analysis		
D100	125.00	
D60	26.10	
D10	0.02	
Uniformity Coefficient	1300.00	

pgl _{priority}	PARTICLE SIZE DISTRIBUTION	Job Ref	P19012
geotechnical	BS 1377 : Part 2 : 1990 : Clause 9	Borehole / Pit No	TP17
Location	Longview Developments	Sample No	1
		Depth	1.00 m
Soil Description	Clayey sandy GRAVEL with high cobble content	Sample type	В



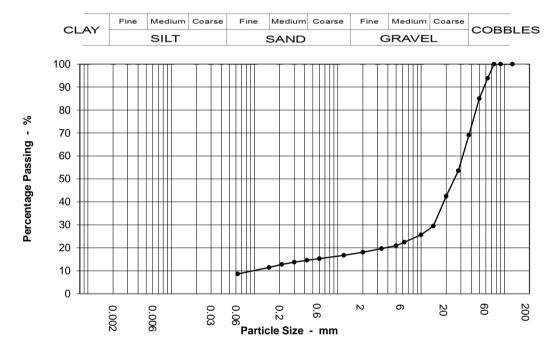
Sievir	Sieving		tation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	93		
63	77		
50	50		
37.5	43		
28	36		
20	31		
14	27		
10	24		
6.3	21		
5	20		
3.35	19		
2	18		
1.18	16		
0.6	15		
0.425	14		
0.3	13		
0.212	12		
0.15	10		
0.063	8		

Test Method	
BS 1377 : Part 2 : 1990	
Sieving Clause 9.3	
Sedimentation	N/A

Sample Proportions		
Cobbles	23.0	
Gravel	60.0	
Sand	10.0	
Silt & Clay	8.0	

Grading Analysis		
D100	90.00	
D60	54.50	
D10	0.15	
Uniformity Coefficient	370.00	

pgl _{priority}	PARTICLE SIZE DISTRIBUTION	Job Ref	P19012
geotechnical	BS 1377 : Part 2 : 1990 : Clause 9	Borehole / Pit No	TP19
Location	Longview Developments	Sample No	1
		Depth	0.60 m
Soil Description	Clayey sandy GRAVEL with low cobble content	Sample type	В



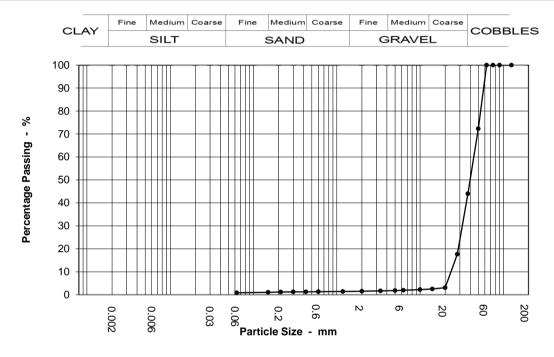
Sievii	ng	Sedimen	tation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	94		
50	85		
37.5	69		
28	54		
20	42		
14	30		
10	26		
6.3	22		
5	21		
3.35	20		
2	18		
1.18	17		
0.6	15		
0.425	15		
0.3	14		
0.212	13		
0.15	11		
0.063	9		

Test Method	
BS 1377 : Part 2 : 1990	
Sieving Clause 9.3	
Sedimentation	N/A

Sample Proportions		
Cobbles	6.0	
Gravel	76.0	
Sand	9.0	
Silt & Clay	9.0	

Grading Analysis		
D100	75.00	
D60	31.60	
D10	0.10	
Uniformity Coefficient	330.00	

pgl _{priority}	PARTICLE SIZE DISTRIBUTION	Job Ref	P19012
geotechnical	BS 1377 : Part 2 : 1990 : Clause 9	Borehole / Pit No	TP20
Location	Longview Developments	Sample No	1
		Depth	0.50 m
Soil Description	Slightly sandy slightly silty GRAVEL	Sample type	В



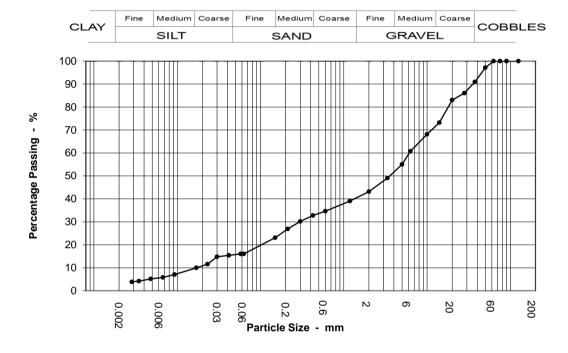
Sievir	ng	Sedimen	tation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	72		
37.5	44		
28	18		
20	3		
14	3		
10	2		
6.3	2		
5	2		
3.35	2		
2	2		
1.18	1		
0.6	1		
0.425	1		
0.3	1		
0.212	1		
0.15	1		
0.063	1		

Test Method		
BS 1377 : Part 2 : 1990		
Sieving	Clause 9.3	
Sedimentation	N/A	

Sample Proportions		
Cobbles	0.0	
Gravel	98.0	
Sand	1.0	
Silt & Clay	1.0	

Grading Analysis		
D100	63.00	
D60	44.10	
D10	23.50	
Uniformity Coefficient 1.90		

pgl _{priority}	PARTICLE SIZE DISTRIBUTION	Job Ref P19012	
geotechnical	BS 1377 : Part 2 : 1990 : Clause 9	Borehole / Pit No	TP21
Location	Longview Developments	Sample No	1
		Depth	1.00 m
Soil Description	Clayey very sandy GRAVEL	Sample type	В



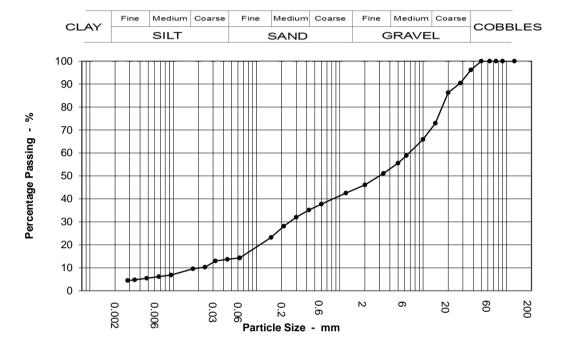
Sieving		Sedimen	tation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.058	16
90	100	0.042	15
75	100	0.030	15
63	100	0.023	12
50	97	0.017	10
37.5	91	0.009	7
28	86	0.007	6
20	83	0.005	5
14	73	0.003	4
10	68	0.003	4
6.3	61	0.002	3
5	55		
3.35	49		
2	43		
1.18	39		
0.6	35		
0.425	33		
0.3	30		
0.212	27		
0.15	23		
0.063	16		

Test Method		
BS 1377 : Part 2 : 1990		
Sieving	Clause 9.5	
Sedimentation	Clause 9.5	

Sample Proportions		
Cobbles	0.0	
Gravel	57.0	
Sand	27.0	
Silt	13.0	
Clay	3.0	

Grading Analysis		
D100 63.00		
D60	6.12	
D10	0.02	
Uniformity Coefficient 360.00		

pgl _{priority}	PARTICLE SIZE DISTRIBUTION	Job Ref P19012	
geotechnical	BS 1377 : Part 2 : 1990 : Clause 9	Borehole / Pit No	TP21
Location	Longview Developments	Sample No	2
		Depth	2.00 m
Soil Description	Clayey very sandy GRAVEL	Sample type	В



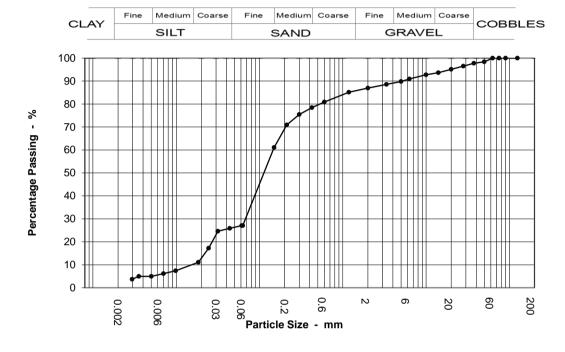
Sieving		Sedimen	tation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.062	14
90	100	0.045	14
75	100	0.032	13
63	100	0.024	10
50	100	0.017	10
37.5	96	0.009	7
28	90	0.007	6
20	86	0.005	5
14	73	0.003	5
10	66	0.003	4
6.3	59	0.002	3
5	56		
3.35	51		
2	46		
1.18	43		
0.6	38		
0.425	35		
0.3	32		
0.212	28		
0.15	23		
0.063	14		

Test Method		
BS 1377 : Part 2 : 1990		
Sieving	Clause 9.5	
Sedimentation	Clause 9.5	

Sample Proportions		
Cobbles 0.0		
Gravel	54.0	
Sand	32.0	
Silt	10.0	
Clay	4.0	

Grading Analysis		
D100 50.00		
D60	6.80	
D10	0.02	
Uniformity Coefficient 310.00		

pglpriority	PARTICLE SIZE DISTRIBUTION	Job Ref	P19012
geotechnical	BS 1377 : Part 2 : 1990 : Clause 9	Borehole / Pit No	TP21
Location	Longview Developments	Sample No	3
		Depth	3.00 m
Soil Description	Gravelly very clayey SAND	Sample type	В



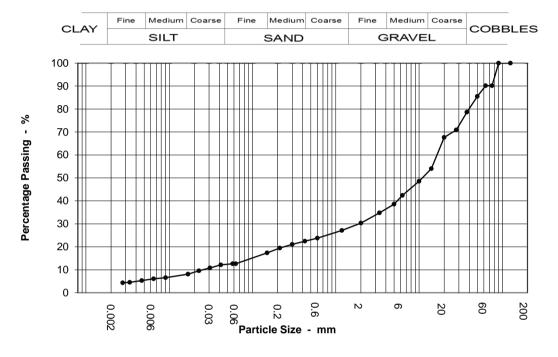
	Sievir	ng	Sedimen	tation
	Particle Size mm	% Passing	Particle Size mm	% Passing
Ī	125	100	0.061	27
l	90	100	0.044	26
l	75	100	0.032	25
l	63	100	0.024	17
l	50	98	0.018	11
l	37.5	98	0.010	7
l	28	97	0.007	6
l	20	95	0.005	5
l	14	94	0.004	5
l	10	93	0.003	4
l	6.3	91	0.002	4
l	5	90		
l	3.35	89		
l	2	87		
l	1.18	85		
l	0.6	81		
l	0.425	78		
l	0.3	75		
I	0.212	71		
I	0.15	61		
I	0.063	27		

Test Method		
BS 1377 : Part 2 : 1990		
Sieving Clause 9.3		
Sedimentation	Clause 9.5	

Sample Proportions		
Cobbles 0.0		
Gravel	13.0	
Sand	60.0	
Silt	23.0	
Clay	4.0	

Grading Analysis		
D100 63.00		
D60	0.15	
D10	0.02	
Uniformity Coefficient 9.50		

pgl _{priority}	PARTICLE SIZE DISTRIBUTION	Job Ref P19012	
geotechnical	BS 1377 : Part 2 : 1990 : Clause 9	Borehole / Pit No	TP22
Location	Longview Developments	Sample No	1
		Depth	0.50 m
Soil Description	Clayey sandy GRAVEL with low cobble content	Sample type	В



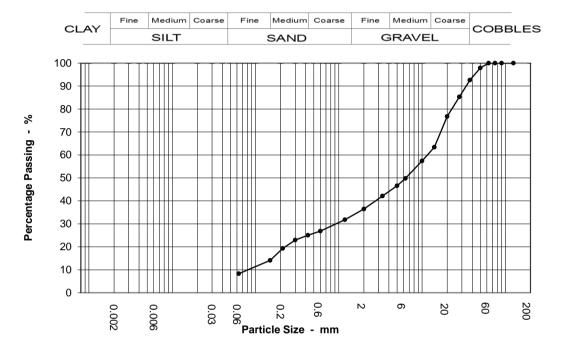
Sievii	ng	Sedimen	tation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.058	13
90	100	0.042	12
75	90	0.031	11
63	90	0.023	10
50	86	0.017	8
37.5	79	0.009	7
28	71	0.006	6
20	68	0.005	5
14	54	0.003	5
10	49	0.003	4
6.3	42	0.001	4
5	39		
3.35	35		
2	30		
1.18	27		
0.6	24		
0.425	22		
0.3	21		
0.212	19		
0.15	17		
0.063	13		

Test Method		
BS 1377 : Part 2 : 1990		
Sieving	Clause 9.5	
Sedimentation	Clause 9.5	

Sample Proportions		
Cobbles 10.0		
Gravel	60.0	
Sand	18.0	
Silt	9.0	
Clay	4.0	

Grading Analysis		
D100 90.00		
D60	16.40	
D10	0.03	
Uniformity Coefficient 650.00		

pgl _{priority}	PARTICLE SIZE DISTRIBUTION	Job Ref	P19012	
geotechnical	BS 1377 : Part 2 : 1990 : Clause 9	Borehole / Pit No	TP22	
Location	Longview Developments	Sample No	2	
		Depth	1.50 m	
Soil Description	Clayey very sandy GRAVEL	Sample type	В	



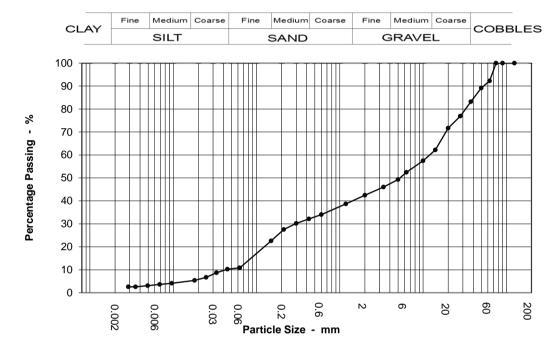
Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	98		
37.5	93		
28	85		
20	77		
14	63		
10	57		
6.3	50		
5	47		
3.35	42		
2	36		
1.18	32		
0.6	27		
0.425	25		
0.3	23		
0.212	19		
0.15	14		
0.063	8		

Test Method		
BS 1377 : Part 2 : 1990		
Sieving Clause 9.3		
Sedimentation	N/A	

Sample Proportions			
Cobbles	0.0		
Gravel	64.0		
Sand	28.0		
Silt & Clay	8.0		

Grading Analysis			
D100	63.00		
D60	11.60		
D10	0.08		
Uniformity Coefficient	140.00		

pgl _{priority}	PARTICLE SIZE DISTRIBUTION	Job Ref	P19012	
BS 1377 : Part 2 : 1990 : Clause 9		Borehole / Pit No	TP23	
Location	Longview Developments	Sample No	3	
		Depth	1.50 m	
Soil Description	Clayey very sandy GRAVEL with low cobble content	Sample type	В	



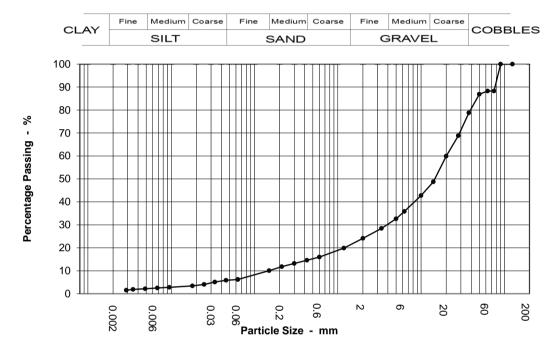
Sieving		Sedimentation		
Particle Size mm	% Passing	Particle Size mm	% Passing	
125	100	0.062	11	
90	100	0.045	10	
75	100	0.033	9	
63	92	0.025	7	
50	89	0.018	5	
37.5	83	0.010	4	
28	77	0.007	4	
20	72	0.005	3	
14	62	0.004	3	
10	57	0.003	3	
6.3	52	0.002	2	
5	49			
3.35	46			
2	42			
1.18	39			
0.6	34			
0.425	32			
0.3	30			
0.212	28			
0.15	23			
0.063	11			

Test Method		
BS 1377 : Part 2 : 1990		
Sieving Clause 9.5		
Sedimentation	Clause 9.5	

Sample Proportions			
Cobbles	8.0		
Gravel	50.0		
Sand	32.0		
Silt	9.0		
Clay	2.0		

Grading Analysis			
D100	75.00		
D60	12.00		
D10	0.04		
Uniformity Coefficient	280.00		

PARTICLE SIZE DISTRIBUTION BS 1377 : Part 2 : 1990 : Clause 9		Job Ref	P19012	
		Borehole / Pit No	TP24	
Location	Longview Developments	Sample No	1	
		Depth	1.00 m	
Soil Description	Clayey very sandy GRAVEL with medium cobble content	Sample type	В	



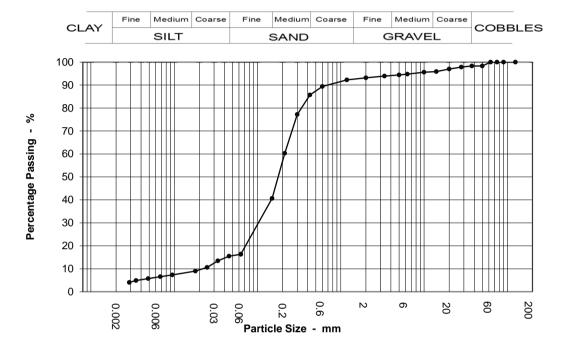
Sieving		Sedimentation		
Particle Size mm	% Passing	Particle Size mm	% Passing	
125	100	0.063	6	
90	100	0.045	6	
75	88	0.033	5	
63	88	0.025	4	
50	87	0.018	3	
37.5	79	0.010	3	
28	69	0.007	2	
20	60	0.005	2	
14	49	0.003	2	
10	43	0.003	2	
6.3	36	0.002	1	
5	33			
3.35	28			
2	24			
1.18	20			
0.6	16			
0.425	15			
0.3	13			
0.212	12			
0.15	10			
0.063	6			

Test Method				
BS 1377 : Part 2 : 1990				
Sieving				
Sedimentation	Clause 9.5			

Sample Proportions				
Cobbles	12.0			
Gravel	64.0			
Sand	18.0			
Silt	5.0			
Clay	1.0			

Grading Analysis			
D100	90.00		
D60	20.10		
D10	0.15		
Uniformity Coefficient	130.00		

pglpriority	PARTICLE SIZE DISTRIBUTION	Job Ref	P19012
geotechnical	BS 1377 : Part 2 : 1990 : Clause 9	Borehole / Pit No	TP24
Location	Longview Developments	Sample No	2
		Depth	2.50 m
Soil Description	Clayey gravelly SAND	Sample type	В



Sievii	ng	Sedimen	tation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.063	16
90	100	0.045	15
75	100	0.033	13
63	100	0.025	11
50	98	0.018	9
37.5	98	0.010	7
28	98	0.007	7
20	97	0.005	6
14	96	0.003	5
10	96	0.003	4
6.3	95	0.002	3
5	94		
3.35	94		
2	93		
1.18	92		
0.6	89		
0.425	86		
0.3	77		
0.212	60		
0.15	41		
0.063	16		

Test Method			
BS 1377 : Part 2 : 1990			
Sieving	Clause 9.5		
Sedimentation	Clause 9.5		

Sample Proportions				
Cobbles	0.0			
Gravel	7.0			
Sand	77.0			
Silt	13.0			
Clay	4.0			

Grading Analysis				
D100	63.00			
D60	0.21			
D10	0.02			
Uniformity Coefficient	9.60			

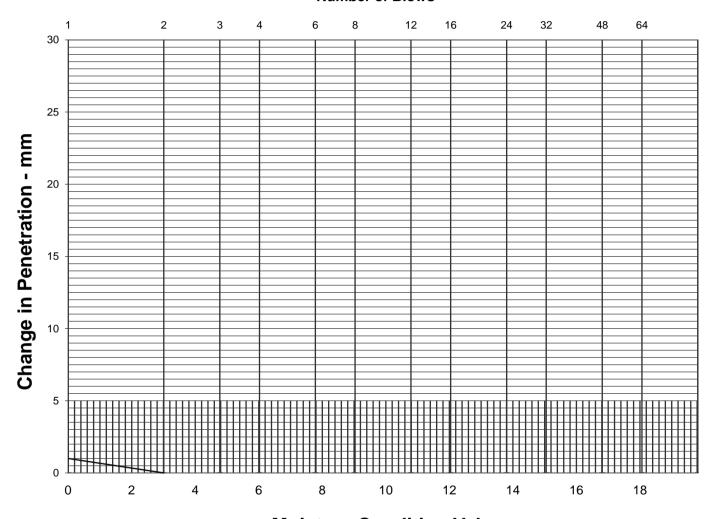
		Dry Density	/ Moisture Content Relation	onship	Job Ref	P19012
			Light Compaction		Borehole / Pit No	TP02
Location			Longview Developments		Sample No	3
Soil Descr	iption		Slightly sandy gravelly SILT		Depth	1.50 m
					Sample Type	В
Test Meth	od	BS1377:F	Part 4:1990, clause 3.4, 2.5kg ramm		Keylab ID Test Reference/No.	PGL12019030425
2.10 2.00 1.90 1.70 1.60					59	% Air Voids % Air Voids % Air Voids
	Material Retai Particle Densi Maximum Dry Optimum Mo	d ned on 37.5 mm Sie ned on 20.0 mm Sie ty - Assumed y Density isture Content	ve % Mg/m³ Mg/m³	Mate	crial used was natural CBR ingle sample tested 4 18 2.65 2.00 10.3	14 16
	Preparation Mould Type Samples Used Material Retai Material Retai Particle Densi Maximum Dry	d ned on 37.5 mm Sie ned on 20.0 mm Sie ty - Assumed y Density isture Content	ve % ve % Mg/m³ Mg/m³ % %	nt, % Mate	erial used was natural CBR ingle sample tested 4 18 2.65	
	Preparation Mould Type Samples Used Material Retai Material Retai Particle Densi Maximum Dry Optimum Mo Natural Mosit	ned on 37.5 mm Sie ned on 20.0 mm Sie ty - Assumed y Density isture Content	ve % ve % Mg/m³ Mg/m³	nt, % Mate	crial used was natural CBR ingle sample tested 4 18 2.65 2.00 10.3	

		Dry Density /	/ Moisture Content F	Relationship	Job Ref	P19012
			Light Compaction	,	Borehole / Pit No	TP03
Location		L	Longview Developments		Sample No	2
Soil Descrip	ption	Ve	ery silty very sandy GRAVE	L	Depth	0.50 m
					Sample Type	В
Test Metho	od	BS1377:Pa	art 4:1990, clause 3.4, 2.5k	g rammer	Keylab ID	PGL12019030430
2.10	-				——— 5 9	% Air Voids % Air Voids % Air Voids
Dry Density, Mg/m ³						
	-	•				
1.70 - 1.60 -	-				1	
1.50 -		8	12 16	5 20	24	28 32
			Moisture	Content, %		
_				•		
Γ	Preparation				Material used was natural	
					Material used was natural	
	Mould Type Samples Used				CBR Single sample tested	
	Mould Type Samples Used Material Retained	d on 37.5 mm Siev		%	CBR Single sample tested	
	Mould Type Samples Used Material Retained Material Retained	d on 20.0 mm Siev	re .	%	CBR Single sample tested 6 14	
	Mould Type Samples Used Material Retained Material Retained Particle Density	d on 20.0 mm Siev - Assumed	Mg/ı	% % m³	CBR Single sample tested 6 14 2.65	
	Mould Type Samples Used Material Retained Material Retained Particle Density Maximum Dry D	d on 20.0 mm Siev - Assumed Density	Mg/r	% % % m³	CBR Single sample tested 6 14 2.65	
	Mould Type Samples Used Material Retained Material Retained Particle Density	d on 20.0 mm Siev - Assumed Density ure Content	Mg/ı	% % % m³	CBR Single sample tested 6 14 2.65	
Operator	Mould Type Samples Used Material Retained Material Retained Particle Density Maximum Dry D Optimum Moiste Natural Mositure	d on 20.0 mm Siev - Assumed Density ure Content e Content	Mg/i	% % % m³	CBR Single sample tested 6 14 2.65 1.80 14.6	Fig

		Dry Density	/ / Moisture Content Rel	ationship	Job Ref	P19012
		2., 20	Light Compaction		Borehole / Pit No	TP03
Location			Longview Developments		Sample No	5
Soil Descr	iption		Sandy very clayey GRAVEL		Depth	1.50 m
					Sample Type	В
Test Metho	od	BS1377:I	Part 4:1990, clause 3.4, 2.5kg r	ammer	Keylab ID	PGL1201903043
2.10				Compacti	on Test Reference/No.	/ % Air Voids
2.00	-					% Air Voids % Air Voids
Dry Density, Mg/m3 1.80 08.1						
1.70						
1.60 ·						
	0	4 8	12 16	20	24 2	28 32
Г			Moisture Co	Г		
	Preparation			Ma 	aterial used was natural	
Ţ	Mould Typ				CBR	
	Samples I	Jsed etained on 37.5 mm Sie	eve %		Single sample tested 16	
		etained on 20.0 mm Sig			27	
ŀ		ensity - Assumed	Mg/m³		2.75	
					1.90	
<u> </u>	Maximum	Dry Density	Mg/m³			
]]]		Dry Density Moisture Content	Mg/m³		15.3	
] []	Optimum					
Operator	Optimum Natural M	Moisture Content ositure Content	%		15.3	Fig

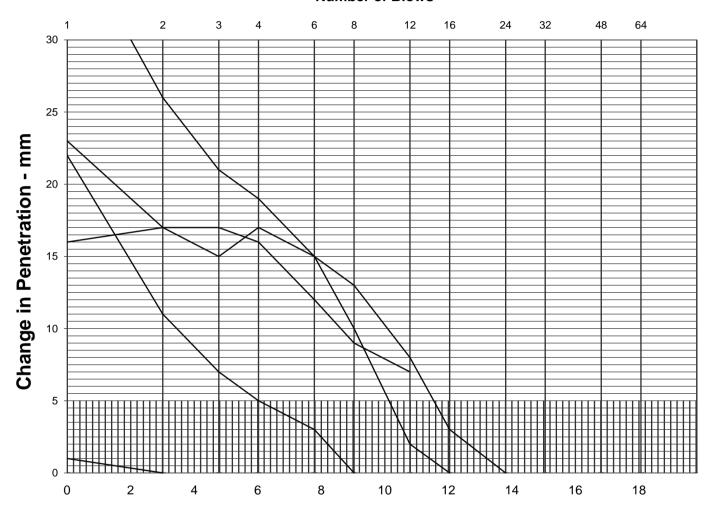
	Dry Density / Moisture Content Re	elationship Job Ref	P19012
	Light Compaction	Borehole / Pit No	TP04
Location	Longview Developments	Sample No	1
Soil Desci	ription Clayey very sandy GRAVEL	Depth	0.50 r
		Sample Type	В
Test Meth	od BS1377:Part 4:1990, clause 3.4, 2.5kg	rammer Keylab ID	PGL1201903043
2.10		Compaction Test Reference/No.	·
2.00		5	% Air Voids % Air Voids 0 % Air Voids
Dry Density, Mg/m3 1.80			
1.80 Dıy De			
1.70			
1.60			
1.50	<u> </u>		
	0 2 4 6 8 Moisture 0	10 12 Content, %	14 16
	Preparation	Material used was natura	ıl
	Mould Type	CBR	
	Samples Used	Single sample tested	
	Material Retained on 37.5 mm Sieve		
	Material Retained on 20.0 mm Sieve Particle Density - Assumed Mg/m	6 12 3 2.75	
	i ande Density - Assumed Mg/m	2./3	
	Maximum Dry Density Mg/m	2.10	
-	Optimum Moisture Content %	11	
		10.74	
	Natural Mositure Content %		
Operator	Remarks	1	Fig

priority geotechnical	Moisture Condition Value	Job Ref	P19012
	BS 1377 : Part 4 : 1990 Clause 5	Borehole / Pit	TP02
Location	Longview Developments	No	4
		Sample No	1
Soil Description	Very sitly very sandy GRAVEL	Sample Type	В
	very sitty very sality SINAVEL	Depth	0.50 m



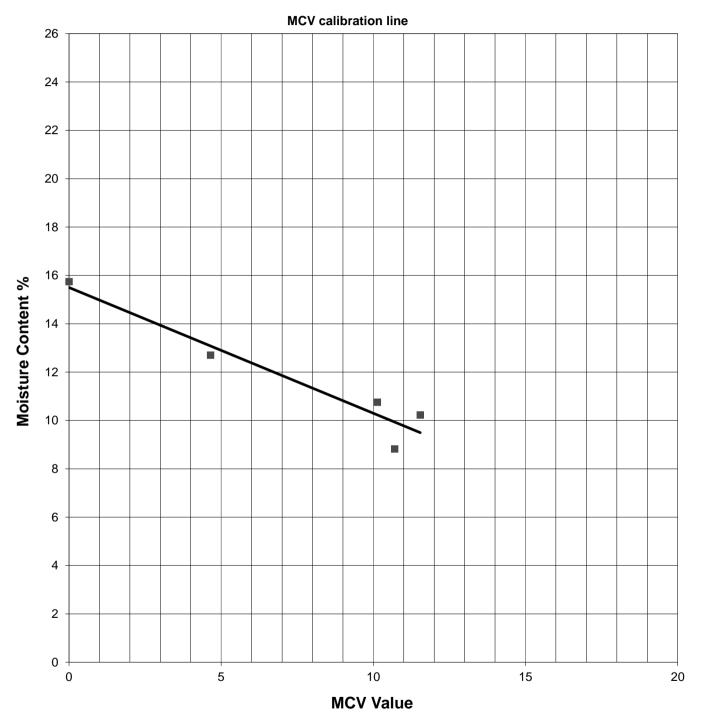
Specimen No		1	2	3	4	5	6
Moisture Condition Value		0.0					
Moisture Content	%	30.42					
Bulk density after compaction	Mg/m³	2.01					
Dry density after compaction	Mg/m³	1.54					
Hand vane strength	kPa						
Method of determining MCV		Steeptest fit line					
Mass retained on 20mm sieve	%	3.4					

pgl _{priority}	Moisture Condition Value	Job Ref	P19012
geotechnical	BS 1377 : Part 4 : 1990 Clause 5	Borehole / Pit No	TP02
Location	Longview Developments	Sample No	3
Soil Description	Slightly sandy gravelly SILT	Sample Type	В
Soil Description	Slightly sandy gravelly SIL1	Depth	1.50 m



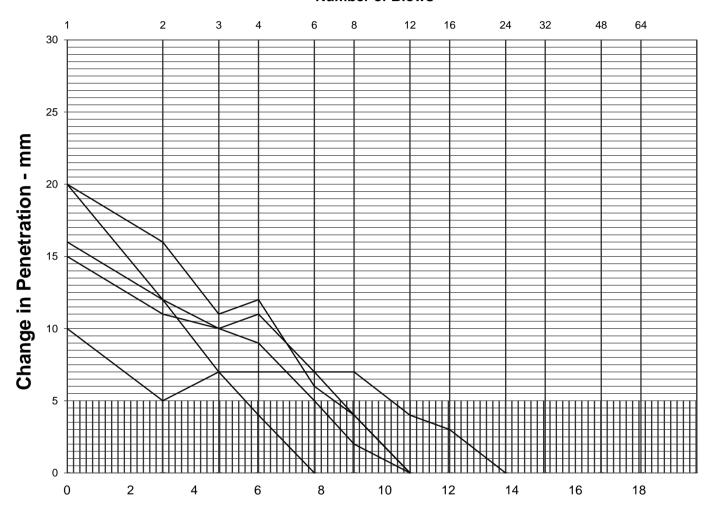
Specimen No		1	2	3	4	5	6
Moisture Condition Value		0.0	10.1	10.7	4.7	11.5	
Moisture Content	%	15.74	10.75	8.82	12.70	10.22	
Bulk density after compaction	Mg/m³	2.17	2.18	2.07	2.18	2.18	
Dry density after compaction	Mg/m³	1.87	1.97	1.90	1.93	1.98	
Hand vane strength	kPa						
Method of determining MCV		Steeptest fit line					
Mass retained on 20mm sieve	%	18.5					

pgl priority	MCV Relationship Graph	Job Ref	P19012					
	BS 1377 : Part 4 : 1990 Clause 5	Borehole / Pit No	TP02					
Location	Longview	Sample No	3					
Soil Description	Slightly sandy gravelly SILT	Sample Type	В					
Soil Description	Slightly Sandy gravelly SIL1	Depth	1.50	m				
MCV calibration line								



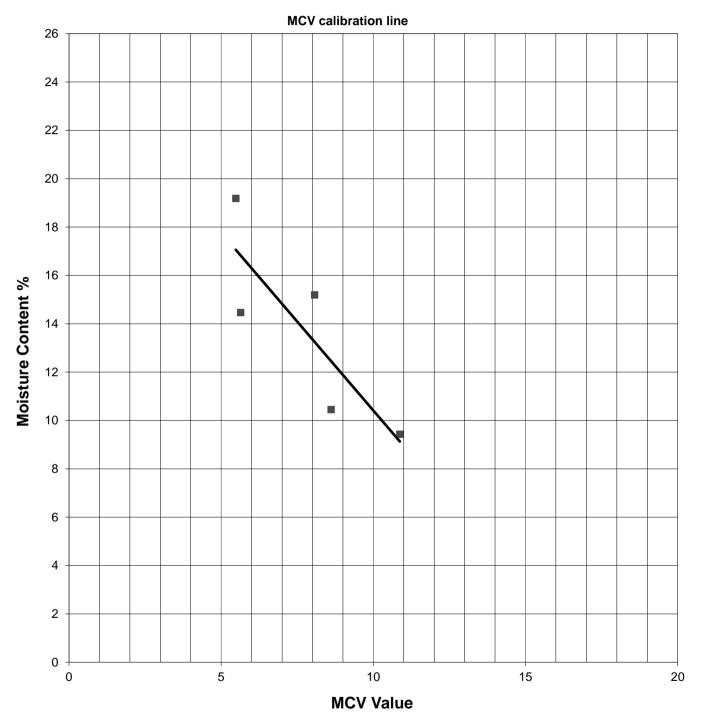
Operator	Checked	Approved	Remarks	Single sample / Separate batches tested

pgl _{priority}	Moisture Condition Value	Job Ref	P19012
geotechnical	BS 1377 : Part 4 : 1990 Clause 5	Borehole / Pit No	TP03
Location	Longview Developments	Sample No	2
Soil Description	Very silty very sandy GRAVEL	Sample Type	В
Soil Description	very silly very salluy GRAVEL	Depth	0.50 m



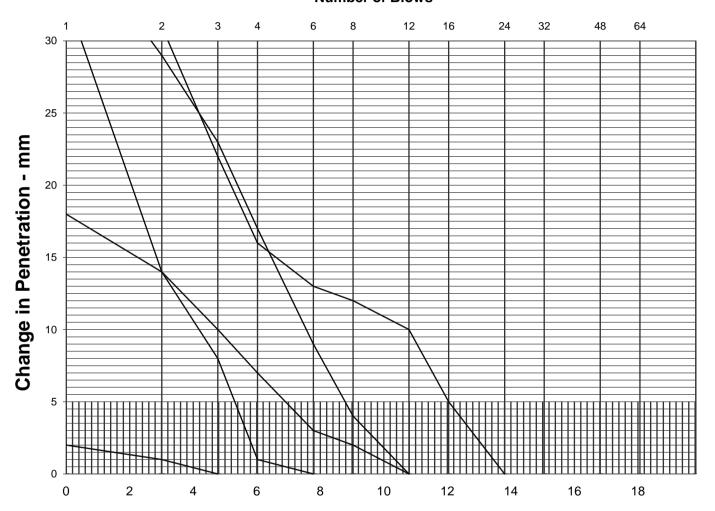
Specimen No		1	2	3	4	5	6
Moisture Condition Value		5.5	8.1	8.6	5.6	10.9	
Moisture Content	%	19.18	15.19	10.45	14.46	9.43	
Bulk density after compaction	Mg/m³	2.05	2.05	1.91	1.97	2.01	
Dry density after compaction	Mg/m³	1.72	1.78	1.73	1.72	1.84	
Hand vane strength	kPa						
Method of determining MCV		Steeptest fit line					
Mass retained on 20mm sieve	%	14.3					

Pgl priority	MCV Relationship Graph	Job Ref	P19012
	BS 1377 : Part 4 : 1990 Clause 5	Borehole / Pit No	TP03
Location	Longview	Sample No	2
0.115	Very silty very sandy GRAVEL	Sample Type	В
Soil Description	very silly very saridy GNAVEE	Depth	0.50 m
20	MCV calibration line		



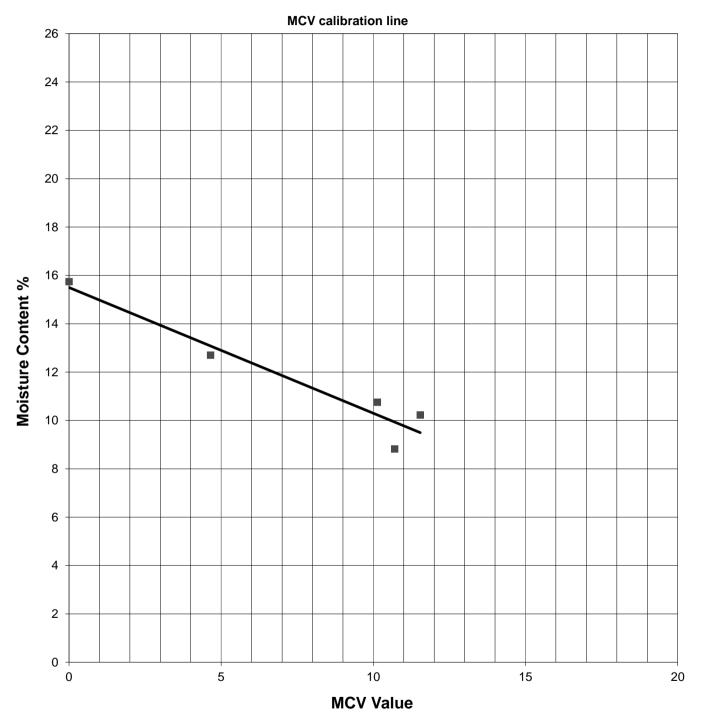
Operator	Checked	Approved	Remarks	Single sample / Separate batches tested

pgl priority geotechnical	Moisture Condition Value	Job Ref	P19012
	BS 1377 : Part 4 : 1990 Clause 5	Borehole / Pit No	TP03
Location	Longview Developments		5
Soil Description	Sandy very clayey GRAVEL	Sample Type	В
3011 Description	Januy Very Glayey GITAVEE	Depth	1.50 m



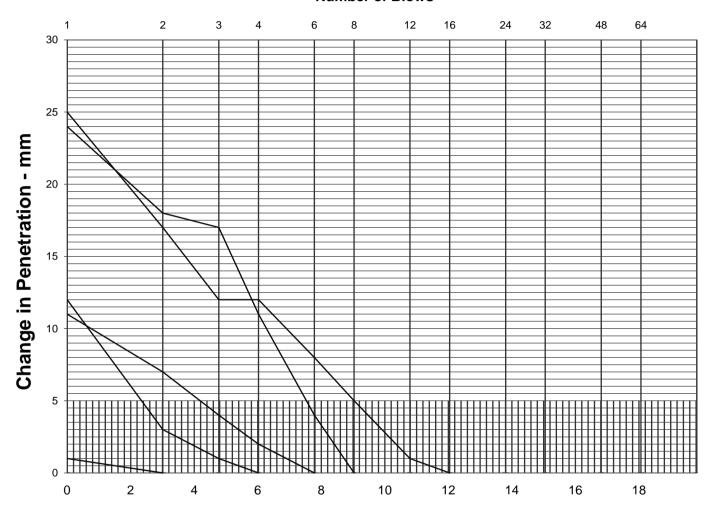
Specimen No		1	2	3	4	5	6
Moisture Condition Value		6.9	8.1	4.4	8.5	0.0	
Moisture Content	%	15.20	14.20	16.18	13.72	17.93	
Bulk density after compaction	Mg/m³						
Dry density after compaction	Mg/m³	0.00	0.00	0.00	0.00	0.00	
Hand vane strength	kPa						
Method of determining MCV		Steeptest fit line					
Mass retained on 20mm sieve	%	26.7					

pgl _{priority}	MCV Relationship Graph	Job Ref	P19012					
geotechnical	BS 1377 : Part 4 : 1990 Clause 5	Borehole / Pit No	TP03					
Location	Longview	Sample No	5					
Soil Description			В					
Soil Description	Sandy very clayey GRAVEL	Depth	1.50 m					
MCV calibration line								



Operator	Checked	Approved	Remarks	Single sample / Separate batches tested

pgl priority geotechnical	Moisture Condition Value	Job Ref	P19012
geotechnical	BS 1377 : Part 4 : 1990 Clause 5	Borehole / Pit	TP04
Location	Longview Developments	No Sample No	1
0.115		Sample Type	В
Soil Description	Clayey very sandy GRAVEL	Depth	0.50 m



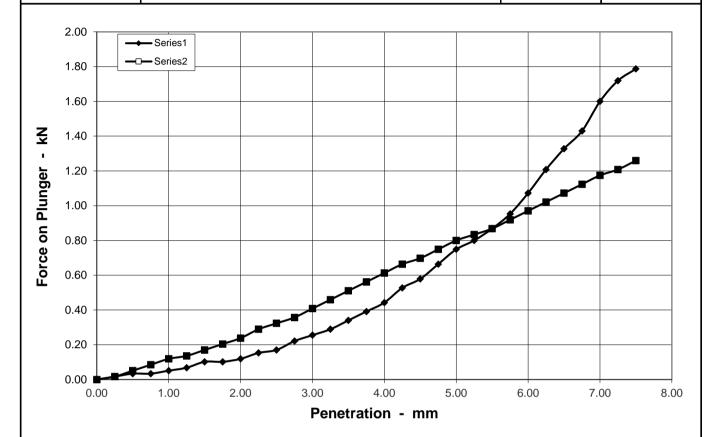
Specimen No		1	2	3	4	5	6
Moisture Condition Value		4.2	7.3	2.3	0.0	7.2	
Moisture Content	%	10.74	9.75	11.64	12.35	10.14	
Bulk density after compaction	Mg/m³	2.29	2.19	2.25	2.25	2.32	
Dry density after compaction	Mg/m³	2.07	2.00	2.02	2.00	2.11	
Hand vane strength	kPa						
Method of determining MCV		Steeptest fit line					
Mass retained on 20mm sieve	%	12.5					

Location Soil Descript												Job Ref Borehole / Pit			P19012 TP04						
Soil Descript	tion			BS 1377 : Part 4 : 1990 Clause 5 Longview							Longview		Longview		Longview		ngview No Sample No			1704	
Soil Descript	tion		0			. ODA	\					Samp	ole Type		В						
				layey ve	ry sandy	/ GRA	VEL					Depth	า	0).50	m					
26 -			T	N	/ICV cal	ibratio	n line	e													
24 -																					
22 -																					
20 -																					
18 -																					
% ^{16 -}																					
Moisture Content %																					
tree G																					
io E 10 -				\	<u> </u>																
8 -																					
6 -																					
4 -																					
2 -																					
0 -	0		5			1	0				15				20						
					N	VCV	Valu	e													
Operator	Checked	Approv	ed	rks Sing	le sample	/ Sepa	rate ba	atches	tested												

P19012 Longview Initial Consumption of Lime

Location	Depth,	рН оС		% Lime							
	m bgl		0.5	1.0	1.5	2.0	2.5	3.0	4.0	5.0	6.0
TP02	1.50	19oC		12.4	12.7	12.8		12.9	12.9	13.0	13.0
		25oC		12.4	12.7	12.8		12.9	12.9	13.0	13.0
TP03	0.50	19oC		12.3	12.6	12.6		12.8	12.8	12.6	12.8
		25oC		12.3	12.6	12.6		12.8	12.8	12.7	12.8
TP04	0.50	19oC	12.2	12.9	13.0	13.0	13.0				
		25oC	12.3	12.9	13.0	13.0	13.0				
TP05	1.00	19oC	12.2	12.6	12.8	12.9	13.0				
		25oC	12.2	12.6	12.8	13.0	13.0				

pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012
geotechnical	BS 13377 : Part 4 : 1990 Clause 7.4	Borehole / Pit No	TP01
Site Name	Site Name Longview Developments		1
		Depth	0.5 m
Soil Description	Silty sandy GRAVEL with low cobble content		



		Method of Compation	
ا ا			
Preparation			
leps	Hammer type		2.5kg Rammer
<u> </u>	Soaking Period	days	
	Amount of Swell	mm	

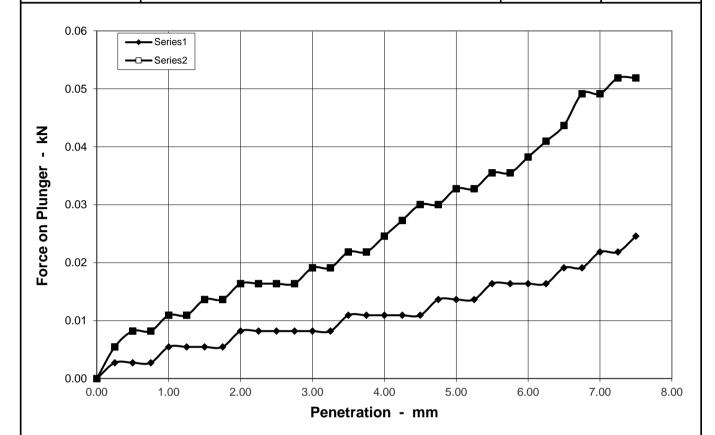
Sample Conditions							
Natural Moisture Content	%	13.0					
Moisture Content - TOP	%	13.1					
Moisture Content - BASE	%	13.4					
Bulk Density	Mg/m³	2.10					
Dry Density	Mg/m³	1.86					

Test Conditions								
Sample Retained on 20 mm sieve	%	25.5						
Seating Load - TOP	N							
Seating Load - BASE	N							
Surcharge	kg	8						

Penetration mm	CBR V	alues %
renetiation min	TOP	BASE
2.5	1.3	2.4
5	3.7	4.0
Accepted CBR	3.7	4.0

	Remarks	

pgl _{priority} CALIFORNIA BEARING RA		Job Ref	P19012
geotechnical	BS 13377 : Part 4 : 1990 Clause 7.4	Borehole / Pit No	TP02
Site Name	Longview Developments	Sample No	1
		Depth	0.5 m
Soil Description	Very sitly very sandy GRAVEL		



		Method of Compation	
ا ا			
Preparation			
leps	Hammer type		2.5kg Rammer
<u> </u>	Soaking Period	days	
	Amount of Swell	mm	

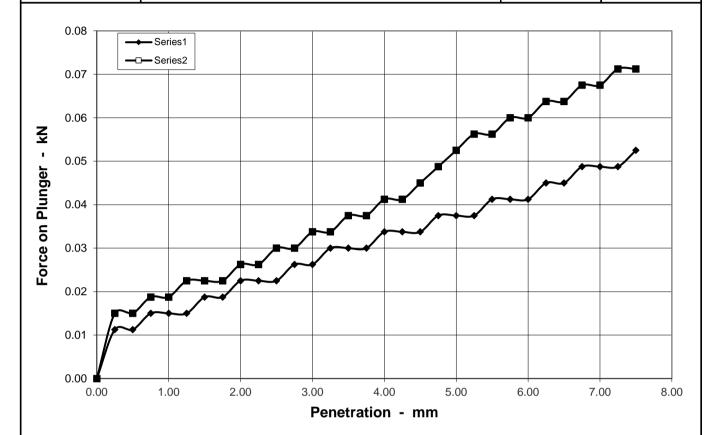
Sample Conditions			
Natural Moisture Content	%	30.0	
Moisture Content - TOP	%	29.8	
Moisture Content - BASE	%	27.1	
Bulk Density	Mg/m³	1.94	
Dry Density	Mg/m³	1.50	

Test Conditions		
Test Conditions	i	
Sample Retained on 20 mm sieve	%	3.4
Seating Load - TOP	Ν	
Seating Load - BASE	N	
Surcharge	kg	8

Penetration mm	CBR Values %	
Penetration min	TOP	BASE
2.5	0.1	0.1
5	0.1	0.2
Accepted CBR	0.1	0.2

	Demonstra	
	Remarks	

pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012
geotechnical	BS 13377 : Part 4 : 1990 Clause 7.4	Borehole / Pit No	TP02
Site Name	Longview Developments	Sample No	3
		Depth	1.5 m
Soil Description	Slightly sandy gravelly SILT		



		Method of Compation	
٦			
Preparation			
гера	Hammer type		2.5kg Rammer
ď	Soaking Period	days	
	Amount of Swell	mm	

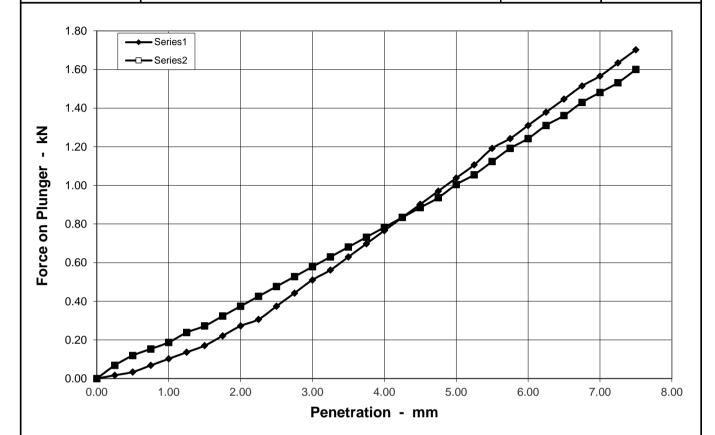
Sample Conditions			
Natural Moisture Content	%	16.0	
Moisture Content - TOP %		15.9	
Moisture Content - BASE	%	14.3	
Bulk Density	Mg/m³	2.19	
Dry Density	Mg/m³	1.89	

Test Conditions		
Sample Retained on 20 mm sieve	%	18.5
Seating Load - TOP	Ν	
Seating Load - BASE	Ν	
Surcharge	kg	8

Penetration mm	CBR Values %	
Penetration min	TOP	BASE
2.5	0.2	0.2
5	0.2	0.3
Accepted CBR	0.2	0.3

	Remarks	

pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012
geotechnical			TP02
Site Name	Longview Developments	Sample No	3
		Depth	1.5 m
Soil Description	Slightly sandy gravelly SILT		



		Method of Compation	
ا ا			
Preparation			
leps	Hammer type		2.5kg Rammer
<u> </u>	Soaking Period	days	
	Amount of Swell	mm	

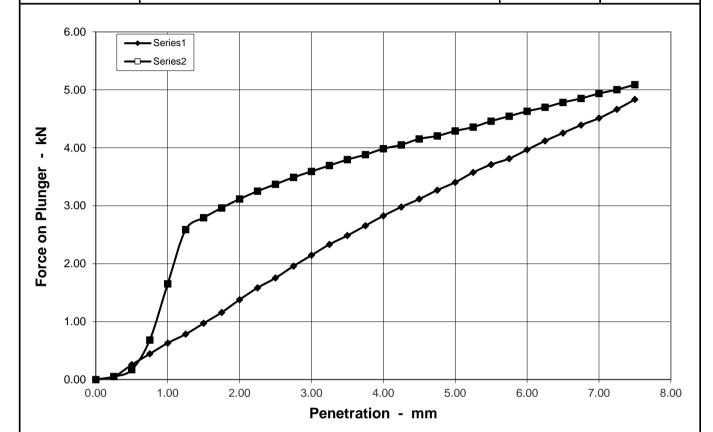
Sample Conditions			
Natural Moisture Content	%	16.0	
Moisture Content - TOP	%	11.7	
Moisture Content - BASE	%	12.1	
Bulk Density	Mg/m³	2.18	
Dry Density	Mg/m³	1.88	

Test Conditions			
Sample Retained on 20 mm sieve	%	18.5	
Seating Load - TOP	Ν		
Seating Load - BASE	Ν		
Surcharge	kg	8	

Penetration mm	CBR Va	alues %
Penetration min	TOP	BASE
2.5	2.8	3.6
5	5.2	5.0
Accepted CBR	5.2	5.0

	Remarks	

pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012
geotechnical			TP02
Site Name	Longview Developments	Sample No	3
		Depth	1.5 m
Soil Description	Slightly sandy gravelly SILT		



		Method of Compation	
ا ا			
Preparation			
leps	Hammer type		2.5kg Rammer
<u> </u>	Soaking Period	days	
	Amount of Swell	mm	

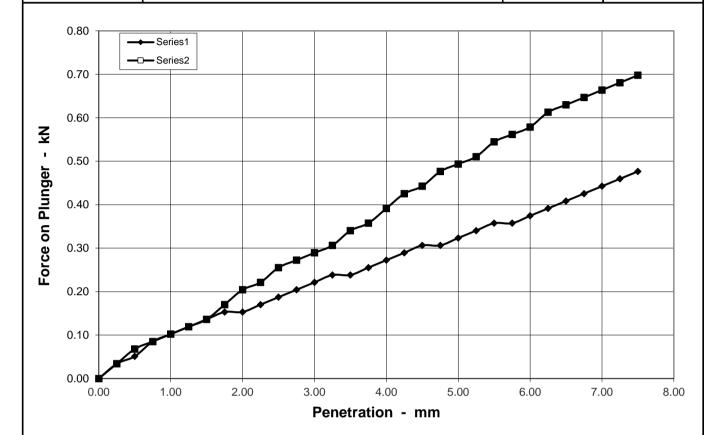
Sample Conditions			
Natural Moisture Content	%	16.0	
Moisture Content - TOP	%	7.8	
Moisture Content - BASE	%	8.4	
Bulk Density	Mg/m³	1.98	
Dry Density	Mg/m³	1.71	

Test Conditions		
Sample Retained on 20 mm sieve	%	18.5
Seating Load - TOP	Ν	
Seating Load - BASE	N	
Surcharge	kg	8

Penetration mm	CBR Values %	
Penetration min	TOP	BASE
2.5	13.3	25.5
5	17.0	21.4
Accepted CBR	17.0	25.5

	Remarks	

pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012	
geotechnical	BS 13377 : Part 4 : 1990 Clause 7.4	Borehole / Pit No	TP02	
Site Name	Longview Developments	Sample No	3	
		Depth	1.5 m	
Soil Description	Slightly sandy gravelly SILT			



		Method of Compation	
l e			
Preparation			
leps	Hammer type		2.5kg Rammer
٦	Soaking Period	days	
	Amount of Swell	mm	

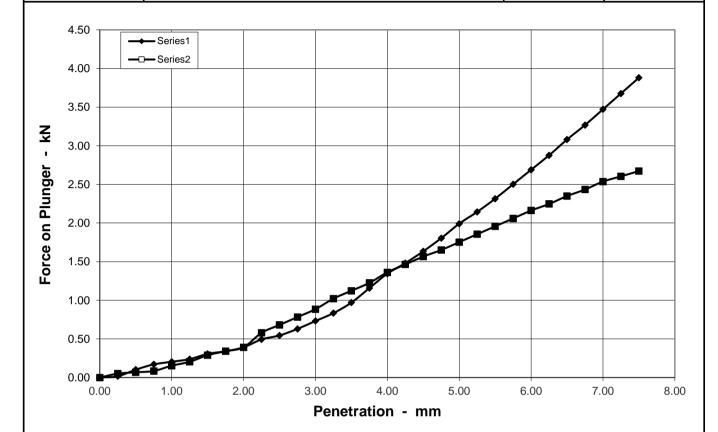
Sample Conditions			
Natural Moisture Content	%	16.0	
Moisture Content - TOP	%	12.6	
Moisture Content - BASE	%	11.7	
Bulk Density	Mg/m³	2.21	
Dry Density	Mg/m³	1.91	

Test Conditions			
Sample Retained on 20 mm sieve	%	18.5	
Seating Load - TOP	N		
Seating Load - BASE	N		
Surcharge	kg	8	

Accepted CBR	1.6	2.5	
5	1.6	2.5	
2.5	1.4	1.9	
Penetration mm	ТОР	BASE	
	CBR Values %		

	Remarks	

pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012	
geotechnical	BS 13377 : Part 4 : 1990 Clause 7.4	Borehole / Pit No	TP02	
Site Name	Longview Developments	Sample No	3	
		Depth	1.5 m	
Soil Description	Slightly sandy gravelly SILT			



		Method of Compation	
٦			
Preparation			
гера	Hammer type		2.5kg Rammer
ď	Soaking Period	days	
	Amount of Swell	mm	

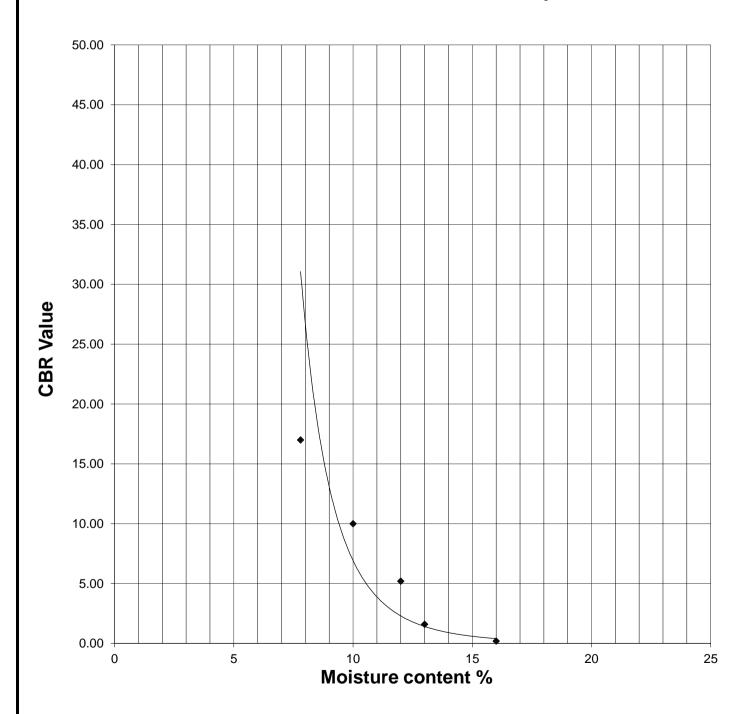
Sample Conditions					
Natural Moisture Content	%	16.0			
Moisture Content - TOP	%	10.2			
Moisture Content - BASE	%	9.9			
Bulk Density	Mg/m³	2.25			
Dry Density	Mg/m³	1.94			

Test Conditions						
Sample Retained on 20 mm sieve	%	18.5				
Seating Load - TOP	Ν					
Seating Load - BASE	Ν					
Surcharge	kg	8				

Penetration mm	CBR Values %	
Penetration min	TOP	BASE
2.5	4.1	5.2
5	10.0	8.8
Accepted CBR	10.0	8.8

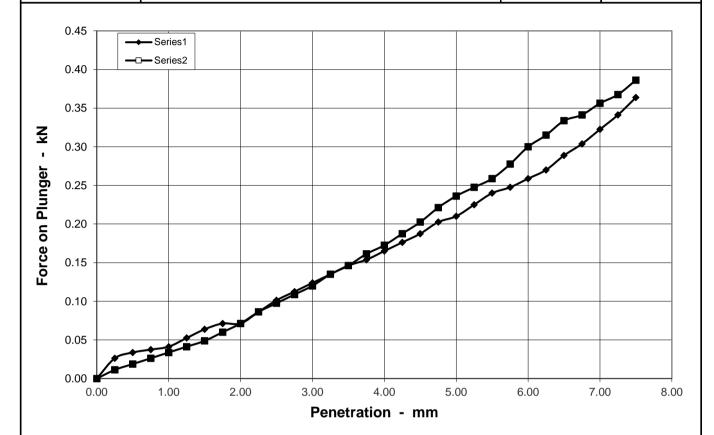
	Remarks	

pglpriority	CALIFORNIA BEARING RATIO RELATIONSHIP		P19012
geotecnnicai	BS 1377 : Part 4 : 1990 Clause 5	Borehole / Pit No	TP02
Location	Longview	Sample No	3
Soil Description	Slightly sandy gravelly SILT	Sample Type	В
Soil Description	Soil Description Slightly Sandy gravelly SIL1		1.50 m



Operator	Checked	Approved

pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012	
geotechnical	BS 13377 : Part 4 : 1990 Clause 7.4	Borehole / Pit No	TP03	
Site Name	Longview Developments	Sample No	2	
		Depth	0.5 m	
Soil Description	Very silty very sandy GRAVEL			



		Method of Compation	
ا ا			
Preparation			
leps	Hammer type		2.5kg Rammer
<u> </u>	Soaking Period	days	
	Amount of Swell	mm	

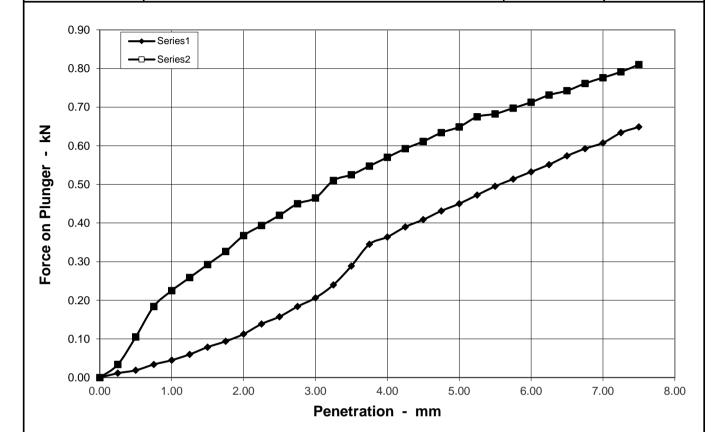
Sample Conditions					
Natural Moisture Content	%	19.0			
Moisture Content - TOP	%	18.0			
Moisture Content - BASE	%	19.7			
Bulk Density	Mg/m³	2.10			
Dry Density	Mg/m³	1.76			

Test Conditions					
Sample Retained on 20 mm sieve	%	14.3			
Seating Load - TOP	Ν				
Seating Load - BASE	N				
Surcharge	kg	8			

Penetration mm	CBR Values %		
Penetration min	TOP	BASE	
2.5	0.8	0.7	
5	1.1	1.2	
Accepted CBR	1.1	1.2	

	Remarks	

pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012	
geotechnical			TP03	
Site Name	Longview Developments	Sample No	2	
		Depth	0.5 m	
Soil Description	Very silty very sandy GRAVEL			



		Method of Compation	
۾			
Preparation			
Leba	Hammer type		2.5kg Rammer
<u>-</u>	Soaking Period	days	
	Amount of Swell	mm	

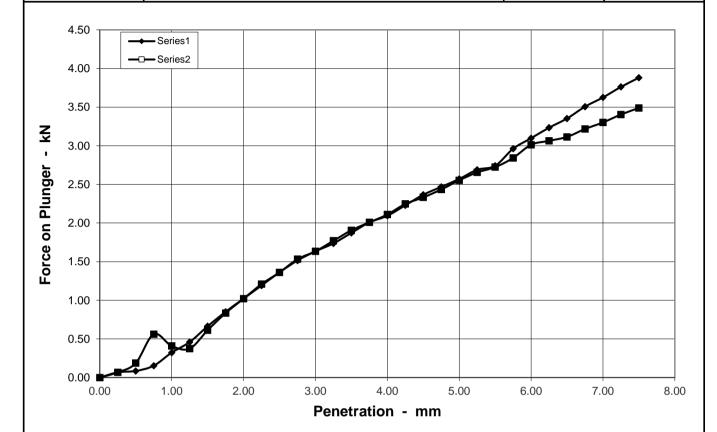
Sample Conditions				
Natural Moisture Content	%	19.0		
Moisture Content - TOP	%	16.6		
Moisture Content - BASE	%	14.7		
Bulk Density	Mg/m³	2.06		
Dry Density	Mg/m³	1.73		

Test Conditions				
Sample Retained on 20 mm sieve	%	14.3		
Seating Load - TOP	Ν			
Seating Load - BASE	N			
Surcharge	kg	8		

Penetration mm	CBR Va	alues %
Penetration min	TOP	BASE
2.5	1.2	3.2
5	2.3	3.2
Accepted CBR	2.3	3.2

	Remarks	

pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012	
geotechnical			TP03	
Site Name	Longview Developments	Sample No	2	
		Depth	0.5 m	
Soil Description	Very silty very sandy GRAVEL			



		Method of Compation	
ا ا			
Preparation			
leps	Hammer type		2.5kg Rammer
<u> </u>	Soaking Period	days	
	Amount of Swell	mm	

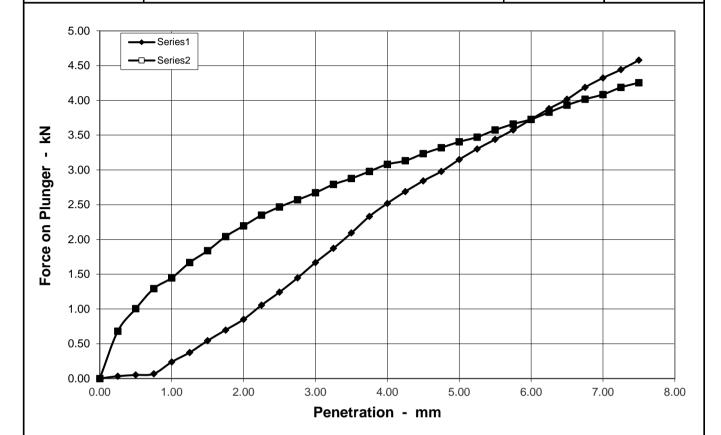
Sample Conditions				
Natural Moisture Content	%	19.0		
Moisture Content - TOP	%	14.1		
Moisture Content - BASE	%	11.9		
Bulk Density	Mg/m³	1.96		
Dry Density	Mg/m³	1.65		

Test Conditions				
Sample Retained on 20 mm sieve	%	14.3		
Seating Load - TOP	Ν			
Seating Load - BASE	N			
Surcharge	kg	8		

Penetration mm	CBR Values %		
Penetration min	TOP	BASE	
2.5	10.3	10.3	
5	12.9	12.8	
Accepted CBR	12.9	12.8	

	Remarks	

pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012
BS 13377 : Part 4 : 1990 Clause 7.4	Borehole / Pit No	TP03	
Site Name	Longview Developments	Sample No	2
		Depth	0.5 m
Soil Description	Very silty very sandy GRAVEL		



		Method of Compation	
ا ا			
Preparation			
leps	Hammer type		2.5kg Rammer
<u> </u>	Soaking Period	days	
	Amount of Swell	mm	

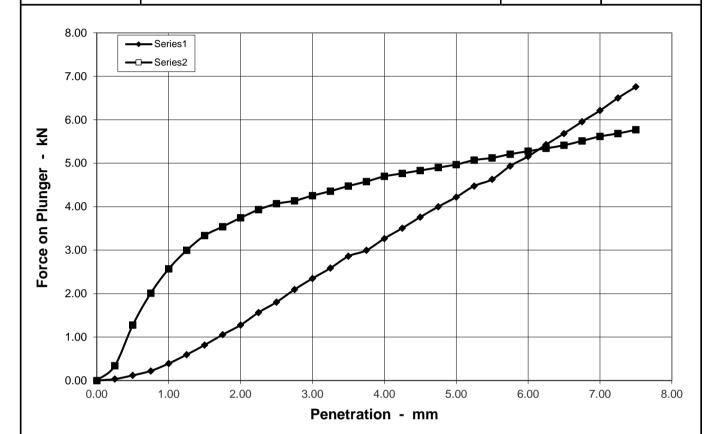
Sample Conditions				
Natural Moisture Content	%	19.0		
Moisture Content - TOP	%	14.8		
Moisture Content - BASE	%	13.7		
Bulk Density	Mg/m³	2.07		
Dry Density	Mg/m³	1.74		

Test Conditions				
Sample Retained on 20 mm sieve	%	14.3		
Seating Load - TOP	N			
Seating Load - BASE	N			
Surcharge	kg	8		

Penetration mm	CBR Values %		
Penetration min	TOP	BASE	
2.5	9.4	18.7	
5	15.7	17.0	
Accepted CBR	15.7	18.7	

	Demonstra	
	Remarks	

pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012	
geotechnical	BS 13377 : Part 4 : 1990 Clause 7.4	Borehole / Pit No	TP03	
Site Name	Longview Developments	Sample No	2	
		Depth	0.5 m	
Soil Description	Very silty very sandy GRAVEL			



		Method of Compation	
٦			
Preparation			
leb	Hammer type		2.5kg Rammer
<u>-</u>	Soaking Period	days	
	Amount of Swell	mm	

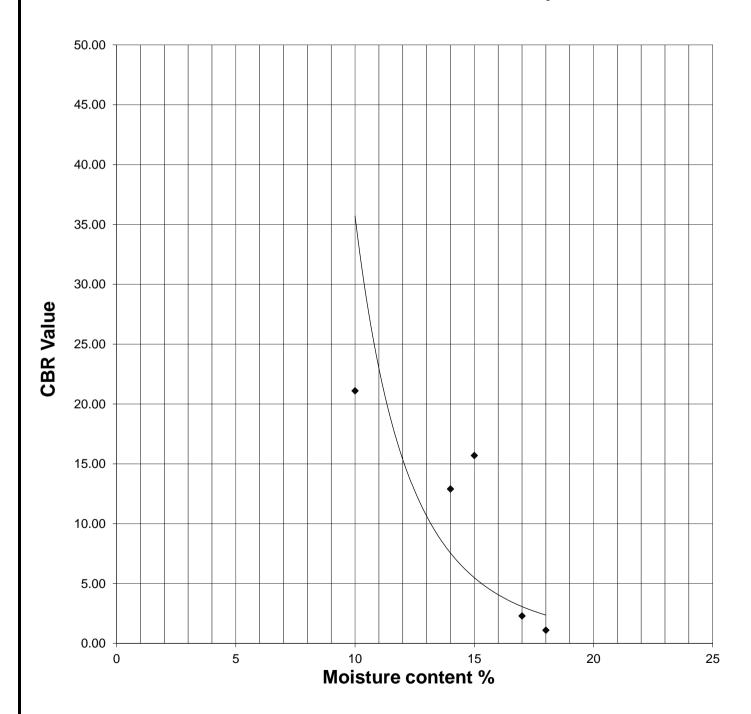
Sample Conditions				
Natural Moisture Content	%	19.0		
Moisture Content - TOP	%	10.2		
Moisture Content - BASE	%	10.3		
Bulk Density	Mg/m³	2.03		
Dry Density	Mg/m³	1.70		

Test Conditions				
Sample Retained on 20 mm sieve	%	14.3		
Seating Load - TOP	Ν			
Seating Load - BASE	Ν			
Surcharge	kg	8		

Accepted CBR	21.1	30.8
5	21.1	24.8
2.5	13.7	30.8
Penetration mm	TOP	BASE
	CBR Va	alues %

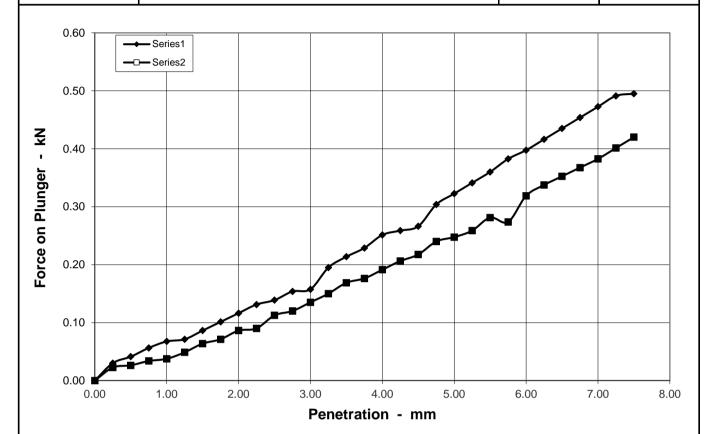
	Remarks	

pglpriority	CALIFORNIA BEARING RATIO RELATIONSHIP	Job Ref P19012	
geotecnnicai	BS 1377 : Part 4 : 1990 Clause 5	Borehole / Pit No	TP03
Location	Longview	Sample No	2
Sail Description	Very silty very sandy GRAVEL	Sample Type	В
Soil Description	very sally sally SKAVEL	Depth	0.50 m



Operator Checked	Approved

pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012
geotechnical	BS 13377 : Part 4 : 1990 Clause 7.4	Borehole / Pit No	TP03
Site Name	Longview Developments	Sample No	5
		Depth	1.5 m
Soil Description	Description Sandy very clayey GRAVEL		



		Method of Compation	
ation			
Preparation	Hammer type		2.5kg Rammer
<u>ا</u> ح	Soaking Period	days	
	Amount of Swell	mm	

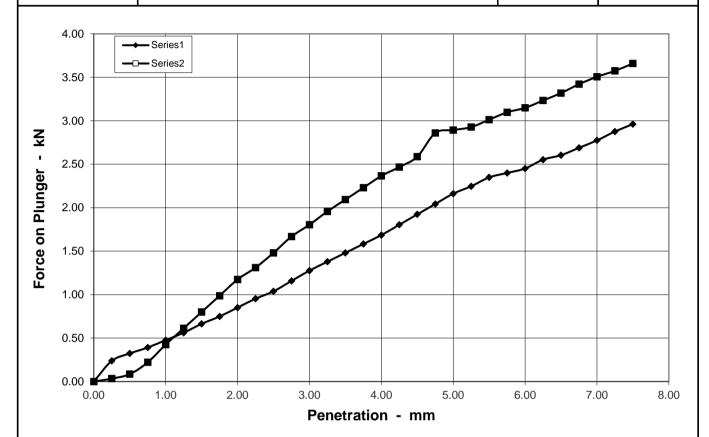
Sample Conditions					
Natural Moisture Content	%	15.0			
Moisture Content - TOP	%	16.4			
Moisture Content - BASE	%	15.2			
Bulk Density	Mg/m³	2.17			
Dry Density	Mg/m³	1.89			

Test Conditions					
Sample Retained on 20 mm sieve	%	26.7			
Seating Load - TOP	N				
Seating Load - BASE	N				
Surcharge	kg	8			

Penetration mm	CBR Values %		
Penetration min	TOP	BASE	
2.5	1.1	0.9	
5	1.6	1.2	
Accepted CBR	1.6	1.2	

	Remarks	

pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012
geotechnical	BS 13377 : Part 4 : 1990 Clause 7.4	Borehole / Pit No	TP03
Site Name	Longview Developments	Sample No	5
		Depth	1.5 m
Soil Description	Description Sandy very clayey GRAVEL		



		Method of Compation	
٦			
Preparation			
leb	Hammer type		2.5kg Rammer
<u>-</u>	Soaking Period	days	
	Amount of Swell	mm	

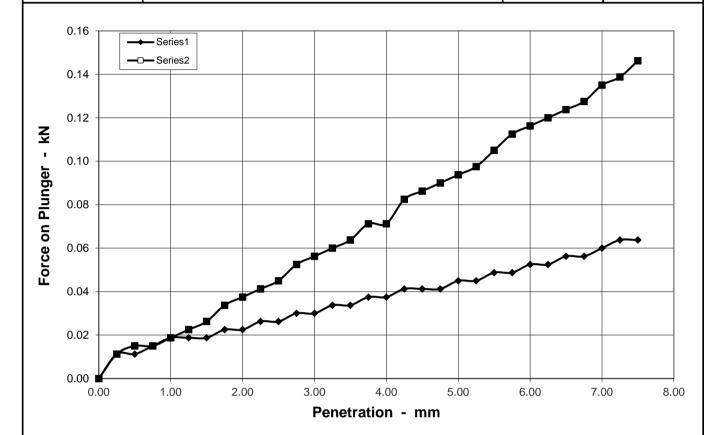
Sample Conditions					
Natural Moisture Content	%	15.0			
Moisture Content - TOP	%	14.1			
Moisture Content - BASE	%	13.0			
Bulk Density	Mg/m³	2.11			
Dry Density	Mg/m³	1.84			

Test Conditions		
Sample Retained on 20 mm sieve	%	26.7
Seating Load - TOP	N	
Seating Load - BASE	N	
Surcharge	kg	8

Penetration mm	CBR Values %		
Penetration min	TOP	BASE	
2.5	7.9	11.2	
5	10.8	14.5	
Accepted CBR	10.8	14.5	

	Remarks	

pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012	
geotechnical		Borehole / Pit No	TP03	
Site Name	Longview Developments	Sample No	5	
		Depth	1.5 m	
Soil Description	Sandy very clayey GRAVEL			



		Method of Compation	
٦			
Preparation			
	Hammer type		2.5kg Rammer
	Soaking Period	days	
	Amount of Swell	mm	

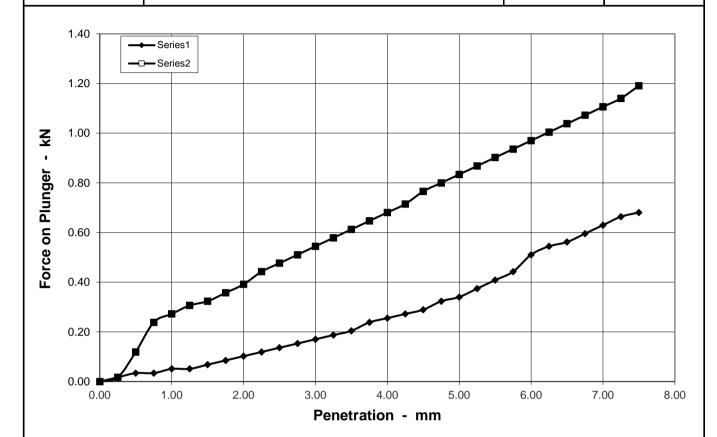
Sample Conditions				
Natural Moisture Content	%	15.0		
Moisture Content - TOP	19.9			
Moisture Content - BASE	%	15.8		
Bulk Density	Mg/m³	2.12		
Dry Density	Mg/m³	1.84		

Test Conditions		
Sample Retained on 20 mm sieve	%	2.7
Seating Load - TOP	Ν	
Seating Load - BASE	Ν	
Surcharge	kg	8

Penetration mm	CBR Values %		
Penetration min	TOP	BASE	
2.5	0.2	0.3	
5	0.2	0.5	
Accepted CBR	0.2	0.5	

	Remarks	

	pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012
ľ	geotechnical	Borehole / Pit No	TP03	
	Site Name	Longview Developments	Sample No	5
			Depth	1.5 m
	Soil Description	Sandy very clayey GRAVEL		



		Method of Compation	
٦			
Preparation			
	Hammer type		2.5kg Rammer
	Soaking Period	days	
	Amount of Swell	mm	

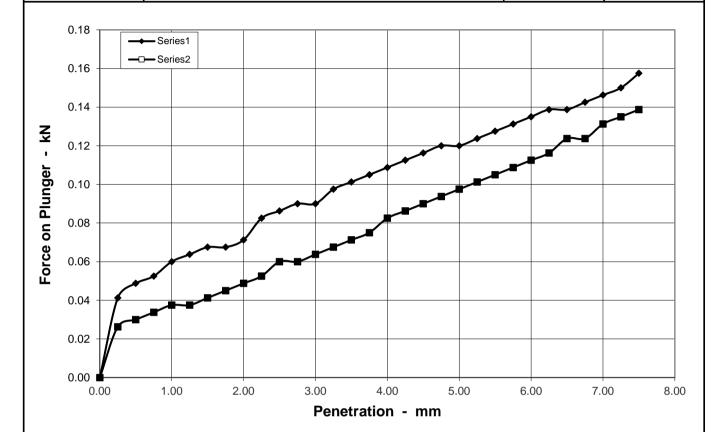
Sample Cond	litions	
Natural Moisture Content	%	15.0
Moisture Content - TOP	%	14.9
Moisture Content - BASE	%	14.1
Bulk Density	Mg/m³	2.07
Dry Density	Mg/m³	1.80

Test Conditions		
Sample Retained on 20 mm sieve	%	26.7
Seating Load - TOP	N	
Seating Load - BASE	N	
Surcharge	kg	8

Penetration mm	CBR Va	alues %
renetiation min	TOP	BASE
2.5	1.0	3.6
5	1.7	4.2
Accepted CBR	1.7	4.2

	Remarks	

pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012
geotechnical	BS 13377 : Part 4 : 1990 Clause 7.4	Borehole / Pit No	TP03
Site Name	Longview Developments	Sample No	5
		Depth	1.5 m
Soil Description	Sandy very clayey GRAVEL		



		Method of Compation	
٦			
Preparation			
leb	Hammer type		2.5kg Rammer
<u>-</u>	Soaking Period	days	
	Amount of Swell	mm	

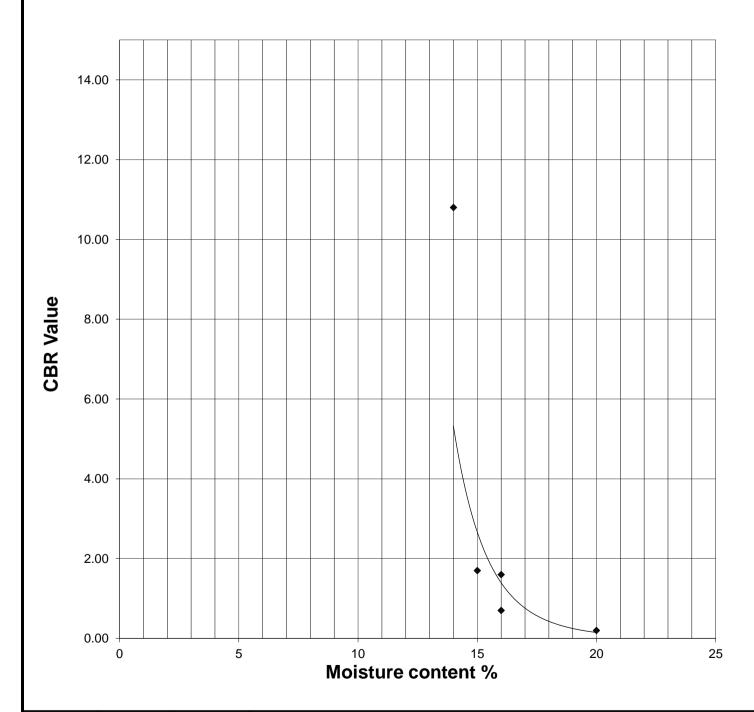
Sample Co	nditions	
Natural Moisture Content	%	15.0
Moisture Content - TOP	%	16.1
Moisture Content - BASE	%	17.9
Bulk Density	Mg/m³	2.17
Dry Density	Mg/m³	1.88

Test Conditions		
Sample Retained on 20 mm sieve	%	26.7
Seating Load - TOP	Ν	
Seating Load - BASE	Ν	
Surcharge	kg	8

Penetration mm	CBR Va	alues %
Penetration min	TOP	BASE
2.5	0.7	0.5
5	0.6	0.5
Accepted CBR	0.7	0.5

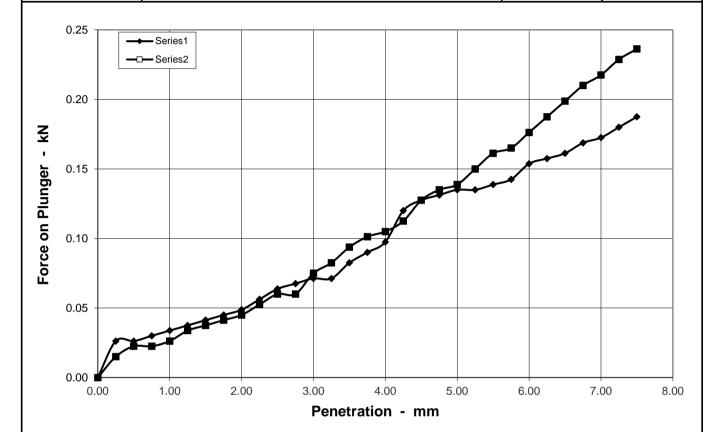
	Remarks	

pglpriority	CALIFORNIA BEARING RATIO RELATIONSHIP	Job Ref	P19012
geotecnnicai	BS 1377 : Part 4 : 1990 Clause 5	Borehole / Pit No	TP03
Location	Longview	Sample No	5
Soil Description	Sandy very clayey GRAVEL	Sample Type	В
Soil Description	Sality very diayey GNAVEL	Depth	1.50 m



Operato	Checke	d Approved

pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012
geotechnical	BS 13377 : Part 4 : 1990 Clause 7.4	Borehole / Pit No	TP04
Site Name	Longview Developments	Sample No	1
		Depth	0.5 m
Soil Description	Clayey very sandy GRAVEL		



	Method of Compation				
ا ا					
Preparation					
leps	Hammer type		2.5kg Rammer		
<u> </u>	Soaking Period	days			
	Amount of Swell	mm			

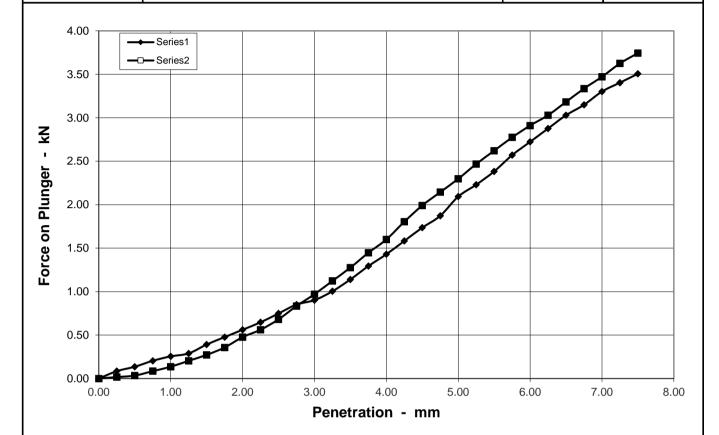
Sample Conditions			
Natural Moisture Content % 11.0			
Moisture Content - TOP	13.7		
Moisture Content - BASE	11.9		
Bulk Density	2.30		
Dry Density	Mg/m³	2.07	

Test Conditions				
Sample Retained on 20 mm sieve	%	12.5		
Seating Load - TOP	N			
Seating Load - BASE	N			
Surcharge	kg	8		

Penetration mm	CBR V	alues %
Penetration min	TOP	BASE
2.5	0.5	0.5
5	0.7	0.7
Accepted CBR	0.7	0.7

	Damania	
	Remarks	

pgl _{priority}	CALIFORNIA BEARING RATIO		P19012	
geotechnical	BS 13377 : Part 4 : 1990 Clause 7.4	Borehole / Pit No	TP04	
Site Name	Longview Developments	Sample No	1	
		Depth	0.5 m	
Soil Description	Clayey very sandy GRAVEL			



	Method of Compation				
ا ا					
Preparation					
leps	Hammer type		2.5kg Rammer		
<u> </u>	Soaking Period	days			
	Amount of Swell	mm			

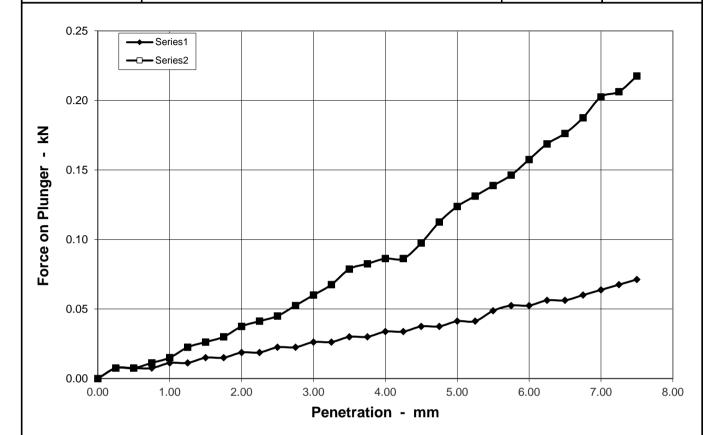
Sample Conditions				
Natural Moisture Content % 11.0				
Moisture Content - TOP	8.7			
Moisture Content - BASE	%	9.7		
Bulk Density	Mg/m³	2.14		
Dry Density Mg/m³ 1.				

Test Conditions				
Sample Retained on 20 mm sieve	%	12.5		
Seating Load - TOP	N			
Seating Load - BASE	N			
Surcharge	kg	8		

Penetration mm	CBR Values %	
Penetration min	TOP	BASE
2.5	5.7	5.2
5	10.5	11.5
Accepted CBR	10.5	11.5

	Remarks	

	pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012
	geotechnical	BS 13377 : Part 4 : 1990 Clause 7.4	Borehole / Pit No	TP04
	Site Name	Longview Developments	Sample No	1
		-	Depth	0.5 m
	Soil Description	Clayey very sandy GRAVEL		



		Method of Compation	
ا ا			
Preparation			
leps	Hammer type		2.5kg Rammer
<u> </u>	Soaking Period	days	
	Amount of Swell	mm	

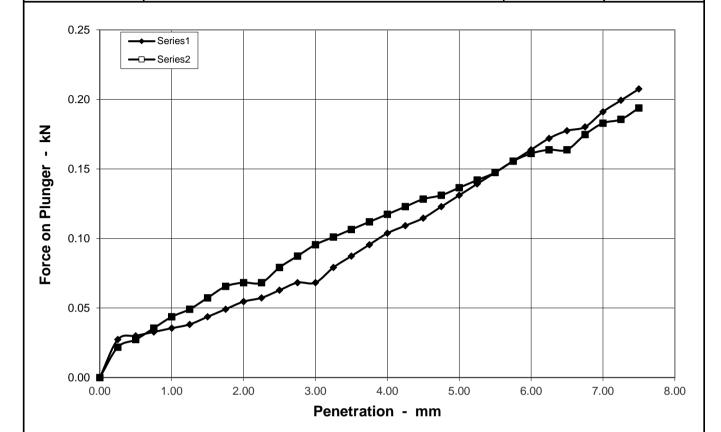
Sample Conditions				
Natural Moisture Content	11.0			
Moisture Content - TOP	%	14.4		
Moisture Content - BASE	%	11.6		
Bulk Density	Mg/m³	2.30		
Dry Density	Mg/m³	2.08		

Test Conditions			
Sample Retained on 20 mm sieve	%	12.5	
Seating Load - TOP	N		
Seating Load - BASE	N		
Surcharge	kg	8	

Penetration mm	CBR Va	alues %
Penetration min	TOP	BASE
2.5	0.2	0.3
5	0.2	0.6
Accepted CBR	0.2	0.6

	Remarks	

	pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012
	geotechnical	BS 13377 : Part 4 : 1990 Clause 7.4	Borehole / Pit No	TP04
	Site Name	Longview Developments	Sample No	1
		-	Depth	0.5 m
	Soil Description	Clayey very sandy GRAVEL		



		Method of Compation	
٦			
Preparation			
leb	Hammer type		2.5kg Rammer
<u>-</u>	Soaking Period	days	
	Amount of Swell	mm	

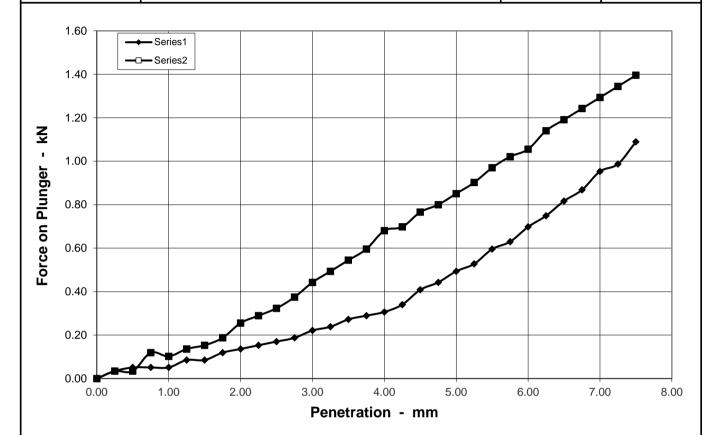
Sample Conditions				
Natural Moisture Content	11.0			
Moisture Content - TOP	%	12.1		
Moisture Content - BASE	%	11.7		
Bulk Density	Mg/m³	2.11		
Dry Density	Mg/m³	1.91		

Test Conditions		
Sample Retained on 20 mm sieve	%	12.5
Seating Load - TOP	N	
Seating Load - BASE	N	
Surcharge	kg	8

Penetration mm	CBR Va	alues %
Penetration min	TOP	BASE
2.5	0.5	0.6
5	0.7	0.7
Accepted CBR	0.7	0.7

	Remarks	

pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012
geotechnical	BS 13377 : Part 4 : 1990 Clause 7.4	Borehole / Pit No	TP04
Site Name	Longview Developments	Sample No	1
		Depth	0.5 m
Soil Description	Clayey very sandy GRAVEL		



		Method of Compation	
uo			
Preparation			
le p	Hammer type		2.5kg Rammer
<u> </u>	Soaking Period	days	
	Amount of Swell	mm	

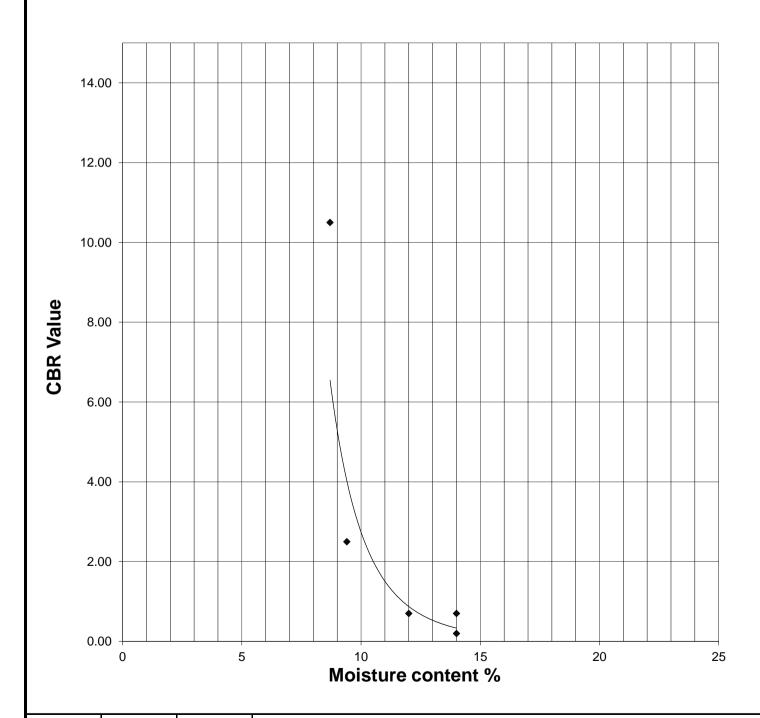
Sample Conditions					
Natural Moisture Content	%	11.0			
Moisture Content - TOP	%	9.4			
Moisture Content - BASE	%	9.6			
Bulk Density	Mg/m³	2.26			
Dry Density	Mg/m³	2.04			

Test Conditions					
Sample Retained on 20 mm sieve	%	12.5			
Seating Load - TOP	Ν				
Seating Load - BASE	Ν				
Surcharge	kg	8			

Penetration mm	CBR Values %	
Penelialion min	TOP	BASE
2.5	1.3	2.4
5	2.5	4.3
Accepted CBR	2.5	4.3

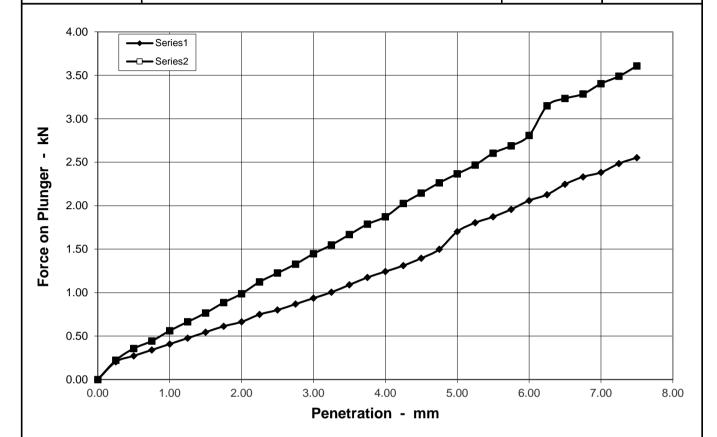
	Remarks	

pgl _{priority}	CALIFORNIA BEARING RATIO RELATIONSHIP		P19012
geotecnnicai	BS 1377 : Part 4 : 1990 Clause 5	Borehole / Pit No	TP04
Location	Longview	Sample No	1
Soil Description	Clayey very sandy GRAVEL	Sample Type	В
Soil Description	Soil Description Clayey very sandy GRAVEL		0.50 m



Operator	Checked	Approved

pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012
geotechnical	BS 13377 : Part 4 : 1990 Clause 7.4	Borehole / Pit No	TP02
Site Name	Longview Developments	Sample No	CBR
		Depth	1.5 m
Soil Description	Slightly sandy gravelly SILT		



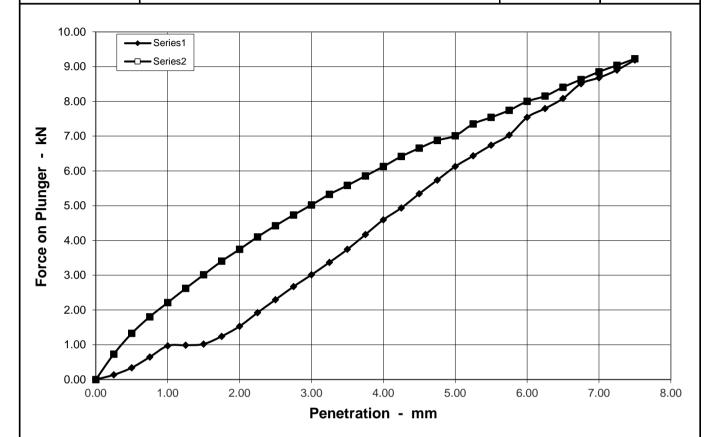
		Method of Compation	
ation			
Preparation	Hammer type		2.5kg Rammer
<u>ا</u> ح	Soaking Period	days	
	Amount of Swell	mm	

Sample Conditions					
Natural Moisture Content	%	15.0			
Moisture Content - TOP	%	16.0			
Moisture Content - BASE	%	15.5			
Bulk Density	Mg/m³	2.23			
Dry Density	Mg/m³	1.93			

Test Conditions						
Sample Retained on 20 mm sieve	%	26.9				
Seating Load - TOP	N					
Seating Load - BASE	N					
Surcharge	kg	8				

Penetration mm	CBR Values %		
Penetration min	TOP	BASE	
2.5	6.1	9.3	
5	8.5	11.8	
Accepted CBR	8.5	11.8	

pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012	
geotechnical	BS 13377 : Part 4 : 1990 Clause 7.4	Borehole / Pit No	TP03	
Site Name	Longview Developments	Sample No	CBR	
		Depth	1.5 m	
Soil Description	Sandy very clayey GRAVEL			



		Method of Compation	
ا ا			
Preparation			
leps	Hammer type		2.5kg Rammer
<u> </u>	Soaking Period	days	
	Amount of Swell	mm	

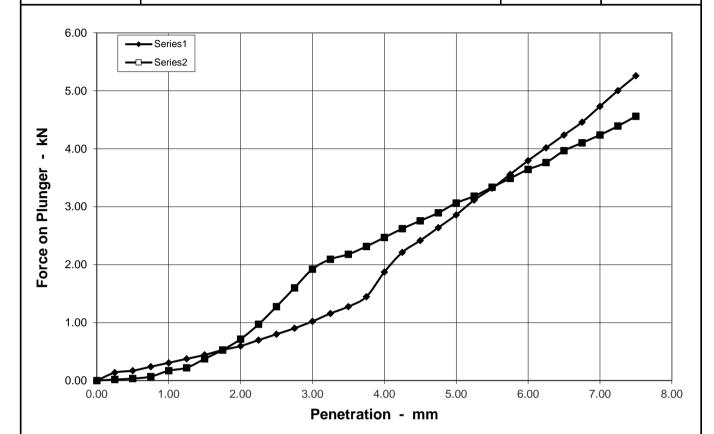
Sample Conditions			
Natural Moisture Content	%	17.0	
Moisture Content - TOP	%	15.4	
Moisture Content - BASE	%	15.4	
Bulk Density	Mg/m³	2.15	
Dry Density	Mg/m³	1.84	

Test Conditions			
Sample Retained on 20 mm sieve	%	12.8	
Seating Load - TOP	N		
Seating Load - BASE	N		
Surcharge	kg	8	

Penetration mm	CBR Values %		
Penetration min	TOP	BASE	
2.5	17.4	33.5	
5	30.6	35.1	
Accepted CBR	30.6	35.1	

	Remarks	
	1% Lime + 2% OPC added	
	The Line 12% of 6 added	

pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012	
geotechnical	BS 13377 : Part 4 : 1990 Clause 7.4	Borehole / Pit No	TP04	
Site Name	Longview Developments	Sample No	CBR	
		Depth	0.5 m	
Soil Description	Sandy very clayey GRAVEL			



	Method of Compation		
ation			
Preparation	Hammer type		2.5kg Rammer
<u>ا</u> ح	Soaking Period	days	
	Amount of Swell	mm	

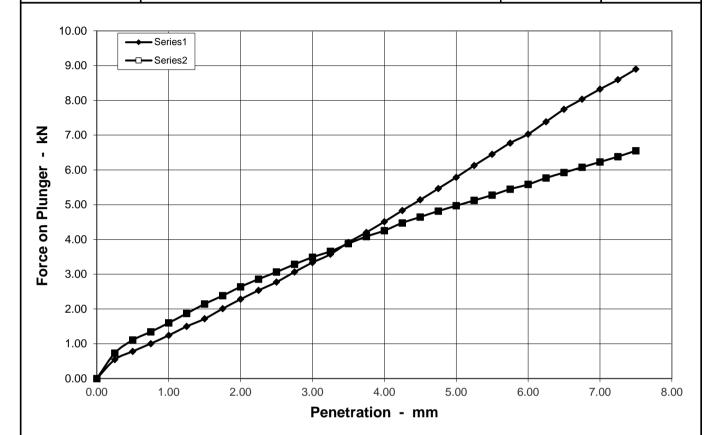
Sample Conditions			
Natural Moisture Content	%	12.0	
Moisture Content - TOP	%	11.1	
Moisture Content - BASE	%	11.2	
Bulk Density	Mg/m³	2.27	
Dry Density	Mg/m³	2.03	

Test Conditions		
Sample Retained on 20 mm sieve	%	21.7
Seating Load - TOP	Ν	
Seating Load - BASE	Ν	
Surcharge	kg	8

Penetration mm	CBR Values %	
Penetration min	TOP	BASE
2.5	6.1	9.7
5	14.3	15.3
Accepted CBR	14.3	15.3

	Remarks	
	1.5% Lime added	

pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012
geotechnical	BS 13377 : Part 4 : 1990 Clause 7.4	Borehole / Pit No	TP05
Site Name	Longview Developments	Sample No	CBR A
		Depth	0.5 m
Soil Description	Slightly sandy slightly gravelly CLAY		



		Method of Compation	
ا ا			
Preparation			
leps	Hammer type		2.5kg Rammer
<u> </u>	Soaking Period	days	
	Amount of Swell	mm	

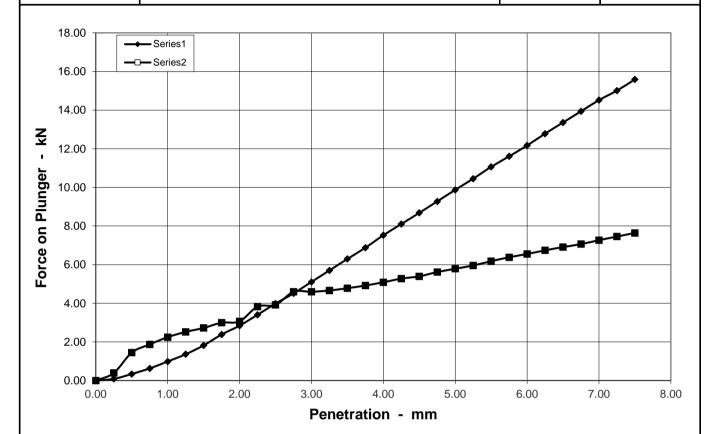
Sample Conditions			
Natural Moisture Content	%	15.0	
Moisture Content - TOP	%	14.0	
Moisture Content - BASE	%	13.2	
Bulk Density	Mg/m³	2.17	
Dry Density	Mg/m³	1.88	

Test Conditions		
Sample Retained on 20 mm sieve	%	2.5
Seating Load - TOP	Ν	
Seating Load - BASE	Ν	
Surcharge	kg	8

Penetration mm	CBR Values %	
Penetration min	TOP	BASE
2.5	21.0	23.2
5	28.9	24.8
Accepted CBR	28.9	24.8

	Remarks	
	1%Lime + 2%OPC added	

pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012
geotechnical	BS 13377 : Part 4 : 1990 Clause 7.4	Borehole / Pit No	TP05
Site Name	Longview Developments	Sample No	CBR B
		Depth	0.5 m
Soil Description	Slightly sandy slightly gravelly CLAY		



		Method of Compation	
ا ا			
Preparation			
leps	Hammer type		2.5kg Rammer
<u> </u>	Soaking Period	days	
	Amount of Swell	mm	

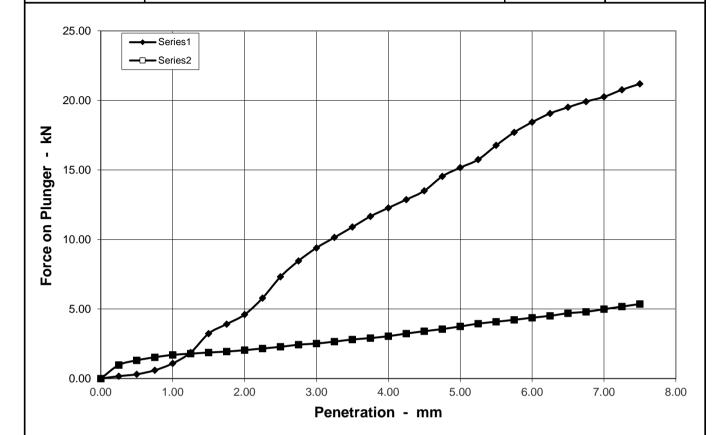
Sample Conditions			
Natural Moisture Content	%	15.0	
Moisture Content - TOP	%	14.6	
Moisture Content - BASE	%	13.3	
Bulk Density	Mg/m³	2.11	
Dry Density	Mg/m³	1.83	

T 10 10					
Test Conditions	Test Conditions				
Sample Retained on 20 mm sieve	%	2.5			
Seating Load - TOP	Ν				
Seating Load - BASE	N				
Surcharge	kg	8			

Penetration mm	CBR Values %	
Penetration min	TOP	BASE
2.5	30.2	29.7
5	49.4	28.9
Accepted CBR	49.4	29.7

	Remarks	
	2% Lime + 2% OPC added	

pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012
geotechnical	BS 13377 : Part 4 : 1990 Clause 7.4	Borehole / Pit No	TP05
Site Name	Longview Developments	Sample No	CBR
		Depth	1 m
Soil Description	Slightly sandy gravelly CLAY		



		Method of Compation	
٦			
Preparation			
leb	Hammer type		2.5kg Rammer
<u>-</u>	Soaking Period	days	
	Amount of Swell	mm	

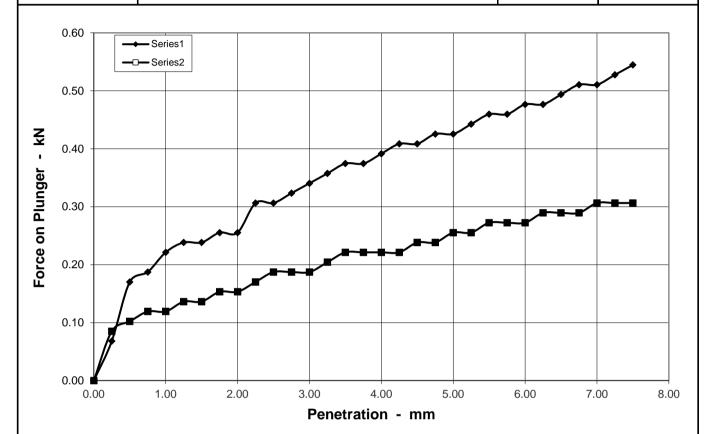
Sample Conditions				
Natural Moisture Content	%	11.0		
Moisture Content - TOP	%	15.1		
Moisture Content - BASE	%	13.1		
Bulk Density	Mg/m³	2.10		
Dry Density	Mg/m³	1.89		

Test Conditions				
Sample Retained on 20 mm sieve	%	28.1		
Seating Load - TOP	N			
Seating Load - BASE	N			
Surcharge	kg	8		

Penetration mm	CBR Values %	
renetiation min	TOP	BASE
2.5	55.4	17.3
5	75.8	18.7
Accepted CBR	75.8	18.7

	Remarks	
	1.5% Lime + 3% OPC added	

pgl _{priority}	CALIFORNIA BEARING RATIO	Job Ref	P19012
geotechnical	BS 13377 : Part 4 : 1990 Clause 7.4	Borehole / Pit No	TP12
Site Name	Longview Developments	Sample No	CBR
		Depth	0.5 m
Soil Description	Sandy very silty GRAVEL with medium cobble content		



		Method of Compation	
ا ا			
Preparation			
leps	Hammer type		2.5kg Rammer
<u> </u>	Soaking Period	days	
	Amount of Swell	mm	

Sample Conditions				
Natural Moisture Content	%	33.0		
Moisture Content - TOP	%	30.5		
Moisture Content - BASE	%	32.5		
Bulk Density	Mg/m³	1.90		
Dry Density	Mg/m³	1.42		

Test Conditions				
Sample Retained on 20 mm sieve	%	37.5		
Seating Load - TOP	N			
Seating Load - BASE	N			
Surcharge	kg	8		

Penetration mm	CBR Values %				
Penetration min	TOP	BASE			
2.5	2.3	1.4			
5	2.1	1.3			
Accepted CBR	2.3	1.4			

	Remarks	
	3%Lime added	



Chemtest Ltd.
Depot Road
Newmarket
CB8 0AL

Tel: 01638 606070 Email: info@chemtest.com

Final Report

Report No.: 19-19751-1

Initial Date of Issue: 17-Jun-2019

Client Priority Geotechnical Ltd

Client Address: Unit 12

Owenacurra Business Park

Midleton County Cork Ireland

Contact(s): Colette Kelly

Project P19012 Longview

Quotation No.: Date Received: 12-Jun-2019

Order No.: 11589 Date Instructed: 12-Jun-2019

No. of Samples: 4

Turnaround (Wkdays): 7 Results Due: 20-Jun-2019

Date Approved: 17-Jun-2019

Approved By:

Details: Martin Dyer, Laboratory Manager



1 TO JOOK! 1 TO OTE Earlighton								
Client: Priority Geotechnical Ltd		Chemtest Job No.:		19-19751	19-19751	19-19751	19-19751	
Quotation No.:		Chemte	est Sam	ple ID.:	841564	841565	841566	841567
		Sa	ample Lo	ocation:	TP03	TP05A	TP05B	TP05
			Sampl	е Туре:	SOIL	SOIL	SOIL	SOIL
		Top Depth (m):		1.5	0.5	0.5	1.0	
		Date Sampled:		10-Jun-2019	10-Jun-2019	10-Jun-2019	10-Jun-2019	
Determinand	Accred.	SOP	Units	LOD				
Moisture	N	2030	%	0.020	12	11	11	10
рН	U	2010		N/A	12.3	12.4	12.6	12.5
Sulphate (2:1 Water Soluble) as SO4	U	2120	g/l	0.010	0.13	0.071	0.042	0.051
Total Sulphur	U	2175 % 0.010		0.048	0.051	0.045	0.069	
Sulphate (Acid Soluble)	U	2430	%	0.010	0.13	0.091	0.088	0.15



Test Methods

SOP	Title	Parameters included	Method summary
2010	pH Value of Soils	рН	pH Meter
2030	Moisture and Stone Content of Soils(Requirement of MCERTS)	Moisture content	Determination of moisture content of soil as a percentage of its as received mass obtained at <37°C.
2120	Water Soluble Boron, Sulphate, Magnesium & Chromium	Boron; Sulphate; Magnesium; Chromium	Aqueous extraction / ICP-OES
2175	Total Sulphur in Soils	Total Sulphur	Determined by high temperature combustion under oxygen, using an Eltra elemental analyser.
2430	Total Sulphate in soils	Total Sulphate	Acid digestion followed by determination of sulphate in extract by ICP-OES.



Report Information

Key

- U UKAS accredited
- M MCERTS and UKAS accredited
- N Unaccredited
- S This analysis has been subcontracted to a UKAS accredited laboratory that is accredited for this analysis
- SN This analysis has been subcontracted to a UKAS accredited laboratory that is not accredited for this analysis
- T This analysis has been subcontracted to an unaccredited laboratory
- I/S Insufficient Sample
- U/S Unsuitable Sample
- N/E not evaluated
 - < "less than"
 - > "greater than"

Comments or interpretations are beyond the scope of UKAS accreditation

The results relate only to the items tested

Uncertainty of measurement for the determinands tested are available upon request

None of the results in this report have been recovery corrected

All results are expressed on a dry weight basis

The following tests were analysed on samples as received and the results subsequently corrected to a dry weight basis TPH, BTEX, VOCs, SVOCs, PCBs, Phenols

For all other tests the samples were dried at < 37°C prior to analysis

All Asbestos testing is performed at the indicated laboratory

Issue numbers are sequential starting with 1 all subsequent reports are incremented by 1

Sample Deviation Codes

- A Date of sampling not supplied
- B Sample age exceeds stability time (sampling to extraction)
- C Sample not received in appropriate containers
- D Broken Container
- E Insufficient Sample (Applies to LOI in Trommel Fines Only)

Sample Retention and Disposal

All soil samples will be retained for a period of 45 days from the date of receipt

All water samples will be retained for 14 days from the date of receipt

Charges may apply to extended sample storage

If you require extended retention of samples, please email your requirements to: <u>customerservices@chemtest.com</u>



Chemtest Ltd.
Depot Road
Newmarket
CB8 0AL

Tel: 01638 606070 Email: info@chemtest.com

Final Report

Report No.:	19-18420-1		
Initial Date of Issue:	10-Jun-2019		
Client	Priority Geotechnical Ltd		
Client Address:	Unit 12 Owenacurra Business Park Midleton County Cork Ireland		
Contact(s):	Colette Kelly		
Project	P19012 Longview		
Quotation No.:		Date Received:	31-May-2019
Order No.:	11589	Date Instructed:	31-May-2019
No. of Samples:	3		
Turnaround (Wkdays):	7	Results Due:	10-Jun-2019
Date Approved:	10-Jun-2019		
Approved By:			
Details:	Martin Dyer, Laboratory Manager		



Client: Priority Geotechnical Ltd	Chemtest Job No.:				19-18420	19-18420	19-18420
Quotation No.:	(Chemte	st Sam	ple ID.:	835453	835454	835455
		Cli	ent Sam	ple ID.:	TP02	TP04	TP12
			Sampl	е Туре:	SOIL	SOIL	SOIL
			Top Dep	oth (m):	1.50	0.50	0.50
	Date Sampled:		29-May-2019	29-May-2019	29-May-2019		
Determinand	Accred.	SOP	Units	LOD			
Moisture	N	2030	%	0.020	11	8.8	23
рН	U	2010		N/A	9.2	11.2	12.2
Sulphate (2:1 Water Soluble) as SO4	U	2120	g/l	0.010	0.29	0.33	0.12
Total Sulphur	U 2175 % 0.010				0.19	0.071	0.45
Sulphate (Acid Soluble)	U	2430	%	0.010	0.28	0.12	0.39



Test Methods

SOP	Title	Parameters included	Method summary
2010	pH Value of Soils	рН	pH Meter
2030	Moisture and Stone Content of Soils(Requirement of MCERTS)	Moisture content	Determination of moisture content of soil as a percentage of its as received mass obtained at <37°C.
2120	Water Soluble Boron, Sulphate, Magnesium & Chromium	Boron; Sulphate; Magnesium; Chromium	Aqueous extraction / ICP-OES
2175	Total Sulphur in Soils	Total Sulphur	Determined by high temperature combustion under oxygen, using an Eltra elemental analyser.
2430	Total Sulphate in soils	Total Sulphate	Acid digestion followed by determination of sulphate in extract by ICP-OES.



Report Information

Key

- U UKAS accredited
- M MCERTS and UKAS accredited
- N Unaccredited
- S This analysis has been subcontracted to a UKAS accredited laboratory that is accredited for this analysis
- SN This analysis has been subcontracted to a UKAS accredited laboratory that is not accredited for this analysis
- T This analysis has been subcontracted to an unaccredited laboratory
- I/S Insufficient Sample
- U/S Unsuitable Sample
- N/E not evaluated
 - < "less than"
 - > "greater than"

Comments or interpretations are beyond the scope of UKAS accreditation

The results relate only to the items tested

Uncertainty of measurement for the determinands tested are available upon request

None of the results in this report have been recovery corrected

All results are expressed on a dry weight basis

The following tests were analysed on samples as received and the results subsequently corrected to a dry weight basis TPH, BTEX, VOCs, SVOCs, PCBs, Phenols

For all other tests the samples were dried at < 37°C prior to analysis

All Asbestos testing is performed at the indicated laboratory

Issue numbers are sequential starting with 1 all subsequent reports are incremented by 1

Sample Deviation Codes

- A Date of sampling not supplied
- B Sample age exceeds stability time (sampling to extraction)
- C Sample not received in appropriate containers
- D Broken Container
- E Insufficient Sample (Applies to LOI in Trommel Fines Only)

Sample Retention and Disposal

All soil samples will be retained for a period of 45 days from the date of receipt

All water samples will be retained for 14 days from the date of receipt

Charges may apply to extended sample storage

If you require extended retention of samples, please email your requirements to: <u>customerservices@chemtest.com</u>





Chemtest Ltd.
Depot Road
Newmarket
CB8 0AL
Tel: 01638 606070

Email: info@chemtest.com

Final Report

Report No.: 19-10538-1

Initial Date of Issue: 04-Apr-2019

Client Priority Geotechnical Ltd

Client Address: Unit 12

Owenacurra Business Park

Midleton County Cork Ireland

Contact(s): Colette Kelly

Project P19012 Longview

Quotation No.: Date Received: 27-Mar-2019

Order No.: 11589 Date Instructed: 27-Mar-2019

No. of Samples: 10

Turnaround (Wkdays): 7 Results Due: 04-Apr-2019

Date Approved: 04-Apr-2019

Approved By:

Details: Robert Monk, Technical Manager



Chemtest Job No.: Client: Priority Geotechnical Ltd 19-10538 19-10538 19-10538 19-10538 19-10538 19-10538 19-10538 19-10538 19-10538 Chemtest Sample ID.: Quotation No.: 800351 800352 800353 800354 800355 800356 800357 800358 800359 Sample Location: TP24 TP05 TP01 TP24 TP19 TP04 TP02 TP22 TP21 Sample Type: SOIL SOIL SOIL SOIL SOIL SOIL SOIL SOIL SOIL Top Depth (m): 1.0 1.0 1.5 2.5 0.6 2.5 2.5 1.5 1.0 Date Sampled: 22-Mar-2019 22-Mar-2019 22-Mar-2019 22-Mar-2019 22-Mar-2019 22-Mar-2019 Determinand Accred. SOP Units LOD Moisture Ν 2030 % 0.020 14 8.1 14 19 12 9.2 14 8.4 9.0 U 2010 N/A 7.5 7.5 7.2 7.7 7.1 7.3 6.2 8.0 7.9 Sulphate (2:1 Water Soluble) as SO4 0.010 U 2120 g/l < 0.010 < 0.010 < 0.010 < 0.010 < 0.010 < 0.010 < 0.010 < 0.010 < 0.010 Total Sulphur U % 0.010 0.020 < 0.010 < 0.010 < 0.010 0.016 < 0.010 0.013 0.015 < 0.010 2175 Sulphate (Acid Soluble) U 2430 0.010 0.028 < 0.010 % 0.014 < 0.010 < 0.010 0.020 < 0.010 < 0.010 < 0.010

Client: Priority Geotechnical Ltd Chemtest Job No.: 19-10538 Quotation No.: Chemtest Sample ID.: 800360 Sample Location: TP6 Sample Type: SOIL Top Depth (m): 1.0 Date Sampled: 22-Mar-2019 Determinand SOP Units LOD Accred. 2030 Moisture Ν % 0.020 14 7.2 U 2010 N/A Sulphate (2:1 Water Soluble) as SO4 U 2120 g/l 0.010 < 0.010 Total Sulphur % 0.010 < 0.010 U 2175 Sulphate (Acid Soluble) < 0.010 2430 0.010



Test Methods

SOP	Title	Parameters included	Method summary
2010	pH Value of Soils	рН	pH Meter
2030	Moisture and Stone Content of Soils(Requirement of MCERTS)	Moisture content	Determination of moisture content of soil as a percentage of its as received mass obtained at <37°C.
2120	Water Soluble Boron, Sulphate, Magnesium & Chromium	Boron; Sulphate; Magnesium; Chromium	Aqueous extraction / ICP-OES
2175	Total Sulphur in Soils	Total Sulphur	Determined by high temperature combustion under oxygen, using an Eltra elemental analyser.
2430	Total Sulphate in soils	Total Sulphate	Acid digestion followed by determination of sulphate in extract by ICP-OES.



Report Information

Key

- U UKAS accredited
- M MCERTS and UKAS accredited
- N Unaccredited
- S This analysis has been subcontracted to a UKAS accredited laboratory that is accredited for this analysis
- SN This analysis has been subcontracted to a UKAS accredited laboratory that is not accredited for this analysis
- T This analysis has been subcontracted to an unaccredited laboratory
- I/S Insufficient Sample
- U/S Unsuitable Sample
- N/E not evaluated
 - < "less than"
 - > "greater than"

Comments or interpretations are beyond the scope of UKAS accreditation

The results relate only to the items tested

Uncertainty of measurement for the determinands tested are available upon request

None of the results in this report have been recovery corrected

All results are expressed on a dry weight basis

The following tests were analysed on samples as received and the results subsequently corrected to a dry weight basis TPH, BTEX, VOCs, SVOCs, PCBs, Phenols

For all other tests the samples were dried at < 37°C prior to analysis

All Asbestos testing is performed at the indicated laboratory

Issue numbers are sequential starting with 1 all subsequent reports are incremented by 1

Sample Deviation Codes

- A Date of sampling not supplied
- B Sample age exceeds stability time (sampling to extraction)
- C Sample not received in appropriate containers
- D Broken Container
- E Insufficient Sample (Applies to LOI in Trommel Fines Only)

Sample Retention and Disposal

All soil samples will be retained for a period of 45 days from the date of receipt

All water samples will be retained for 14 days from the date of receipt

Charges may apply to extended sample storage

If you require extended retention of samples, please email your requirements to: <u>customerservices@chemtest.com</u>





Chemtest Ltd.
Depot Road
Newmarket
CB8 0AL
Tel: 01638 606070

Tel: 01638 606070 Email: info@chemtest.com

Final Report

Report No.: 19-08568-1

Initial Date of Issue: 20-Mar-2019

Client Priority Geotechnical Ltd

Client Address: Unit 12

Owenacurra Business Park

Midleton County Cork Ireland

Contact(s): Colette Kelly

Project P19012 Longview

Quotation No.: Date Received: 08-Mar-2019

Order No.: 11589 Date Instructed: 08-Mar-2019

No. of Samples: 7

Turnaround (Wkdays): 7 Results Due: 18-Mar-2019

Date Approved: 15-Mar-2019

Approved By:

Details: Robert Monk, Technical Manager



Project: P19012 Longview											
Client: Priority Geotechnical Ltd	Chemtest Job No.:		19-08568	19-08568	19-08568	19-08568	19-08568	19-08568	19-08568		
Quotation No.:	(Chemte	est Sam	ple ID.:	790289	790290	790291	790292	790293	790294	790295
	Sample Location:		TP04	TP02	TP02	TP03	TP09	TP09	TP09		
		Sample Type:		SOIL	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL	
		Top Depth (m):		0.5	0.5	1.5	1.5	1.0	2.0	2.9	
	Date Sampled:		05-Mar-2019								
Determinand	Accred.	SOP	Units	LOD							
Moisture	N	2030	%	0.020	9.6	21	11	11	11	11	8.8
рН	U	2010		N/A	7.0	6.5	6.4	6.6	7.0	7.1	7.5
Sulphate (2:1 Water Soluble) as SO4	U	2120	g/l	0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010
Total Sulphur	U	2175	%	0.010	< 0.010	0.013	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010
Sulphate (Acid Soluble)	U	2430	%	0.010	< 0.010	0.022	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010



Test Methods

SOP	Title	Parameters included	Method summary
2010	pH Value of Soils	рН	pH Meter
2030	Moisture and Stone Content of Soils(Requirement of MCERTS)	Moisture content	Determination of moisture content of soil as a percentage of its as received mass obtained at <37°C.
2120	Water Soluble Boron, Sulphate, Magnesium & Chromium	Boron; Sulphate; Magnesium; Chromium	Aqueous extraction / ICP-OES
2175	Total Sulphur in Soils	Total Sulphur	Determined by high temperature combustion under oxygen, using an Eltra elemental analyser.
2430	Total Sulphate in soils	Total Sulphate	Acid digestion followed by determination of sulphate in extract by ICP-OES.



Report Information

Key

- U UKAS accredited
- M MCERTS and UKAS accredited
- N Unaccredited
- S This analysis has been subcontracted to a UKAS accredited laboratory that is accredited for this analysis
- SN This analysis has been subcontracted to a UKAS accredited laboratory that is not accredited for this analysis
- T This analysis has been subcontracted to an unaccredited laboratory
- I/S Insufficient Sample
- U/S Unsuitable Sample
- N/E not evaluated
 - < "less than"
 - > "greater than"

Comments or interpretations are beyond the scope of UKAS accreditation

The results relate only to the items tested

Uncertainty of measurement for the determinands tested are available upon request

None of the results in this report have been recovery corrected

All results are expressed on a dry weight basis

The following tests were analysed on samples as received and the results subsequently corrected to a dry weight basis TPH, BTEX, VOCs, SVOCs, PCBs, Phenols

For all other tests the samples were dried at < 37°C prior to analysis

All Asbestos testing is performed at the indicated laboratory

Issue numbers are sequential starting with 1 all subsequent reports are incremented by 1

Sample Deviation Codes

- A Date of sampling not supplied
- B Sample age exceeds stability time (sampling to extraction)
- C Sample not received in appropriate containers
- D Broken Container
- E Insufficient Sample (Applies to LOI in Trommel Fines Only)

Sample Retention and Disposal

All soil samples will be retained for a period of 45 days from the date of receipt

All water samples will be retained for 14 days from the date of receipt

Charges may apply to extended sample storage

If you require extended retention of samples, please email your requirements to: <u>customerservices@chemtest.com</u>